# LP SOLIDSTART 

BUILDING PRODUCTS
Bulloing products

LPI ${ }^{\circ}$ 18, 20Plus, 32Plus, 36, 42Plus, 52Plus and 56 Series
Floor \& Roof Applications

LP ${ }^{\oplus}$ SolidStart ${ }^{\oplus}$ I-Joists are straighter and more uniform in strength, stiffness and size than traditional lumber, providing a strong, sturdy floor. We offer longer lengths so that ceilings and floors can be designed with fewer pieces, saving time on installation. Other advantages over lumber include lower moisture content, which makes our I-Joists less likely to split, shrink, twist, warp or bow. This means reduced callbacks due to fewer pops and squeaks.

## STRENGTH IN NUMBERS

LP's full range of SolidStart products are designed and manufactured to install easily and work together to provide a strong, sound structure.

For I-Joists, we combine laminated veneer lumber (LVL) or finger-jointed sawn lumber flanges with a web of oriented strand board (OSB) to produce an l-shaped structural member. The webs allow plumbing and wiring to pass through without extra framing, while the flanges resist bending - ideal for long spans in floors, ceilings and roofs.

## LP SolidStart I-JOISTS ARE A BUILDING MATERIAL WITH BUILT-IN ENVIRONMENTAL BENEFITS

- Made of engineered wood substrate, a renewable resource with a reduced environmental impact
- Raw material procurement targets small, fast growing trees
- In LP's manufacturing process, no part of the log goes to waste
- Only low-emitting, safe resins are used as a binder
- Available in longer lengths, reducing the number of pieces needed; this results in more efficient utilization of resources
- Can help you qualify for certification points in a number of leading green building programs



## PEACE-OF-MIND FOR A LIFETIME

If your LP SolidStart I-Joists ever develop problems due to a defect, LP will cover all reasonable repair and/or replacement costs per the conditions of our Lifetime Limited Warranty. Visit LPCorp.com to view our complete warranty, or contact your local LP SolidStart Engineered Wood Products distributor or sales office for an original copy.

## COMPLIANT WITH MAJOR BUILDING CODES

LP SolidStart I-Joists have been evaluated for compliance with major US building codes. Refer to APA product report PR-L238 or ICC-ES evaluation report ESR-1305 for complete product information for LP SolidStart I-Joist. Contact your local LP SolidStart Engineered Wood Products distributor or visit LPCorp.com for the most current code reports.

## LIFETIME LIMITED WARRANTY

LP SolidStart Engineered Wood Products are backed by a lifetime limited warranty. Visit LPCorp.com or call 1.888.820.0325 for a copy of the warranty.
Product Specifications \& Design Values ..... 4
Floor Span Tables ..... 5-7
Uniform Floor Load (PLF) Tables ..... 8-9
Uniform Roof Load (PLF) Tables ..... 10-11
Roof Span Tables: Low Pitch (6:12 or less) ..... 12-13
Roof Span Tables: High Pitch (6:12 to 12:12) ..... 14-15
Load-Bearing Cantilever Tables ..... 16-19
Brick-Ledge Cantilevers ..... 20-21
Web Hole Specifications: Circular Holes ..... 22
Web Hole Specifications: Rectangular Holes ..... 23
Floor Details ..... 24-25
Roof Details ..... 26
Framing Connectors ..... 27
Web Stiffeners, Rim \& Blocking, Nailing ..... 28
LP® ${ }^{\oplus}$ SolidStart ${ }^{\oplus}$ Rim Board ..... 29
Handling and Storage Guidelines ..... 30
Warnings ..... 31


NOTES:

1. LP® SolidStart ${ }^{\ominus}$ I-Joists shall be designed for dry-use conditions only. Dry-use applies to products installed in dry, covered and well ventilated interior conditions in which the equivalent moisture content in lumber will not exceed $16 \%$.
2. Moment and Shear are for normal load duration and shall be adjusted according to code.
3. Moment shall not be increased for repetitive member use
4. Deflection calculations shall include both bending and shear deformations. Deflection for a simple span, uniform load:

$$
\Delta=\frac{22.5 \mathrm{wL}^{4}}{\mathrm{El}}+\frac{\mathrm{wL}^{2}}{\mathrm{~K}}
$$

Where: $\Delta=$ deflection (in) $\quad E I=$ bending stiffness (from table)
$\mathrm{w}=$ uniform load (plf) $\mathrm{K}=$ shear stiffness (from table)
$\mathrm{L}=$ design span (ft)
Equations for other conditions can be found in engineering references

## REACTION AND BEARING CAPACITY

| Series | Depth | End Reaction Capacity ${ }^{1}$ (Ibs) |  |  |  | Interior Reaction Capacity ${ }^{1}$ (Ibs) |  |  |  | Flange Bearing Capacity ${ }^{2}$ (lb/in) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Minimum Bearing (1-1/2") |  | Maximum Bearing (4") |  | Minimum Bearing (3-1/2") |  | Maximum Bearing (5-1/2") |  |  |
|  |  | W/out Stiffeners | With Stiffeners | W/out Stiffeners | With Stiffeners | W/out Stiffeners | With Stiffeners | W/out Stiffeners | With Stiffeners |  |
| LPI 18 | 9-1/2" | 870 | 1025 | 995 | 1130 | 1975 | 2135 | 2205 | 2370 | 955 |
|  | 11-7/8" | 870 | 1145 | 1040 | 1335 | 2095 | 2270 | 2335 | 2545 |  |
|  | 14" | 870 | 1255 | 1080 | 1510 | 2205 | 2395 | 2450 | 2700 |  |
| LPI 20Plus | 9-1/2" | 970 | 1140 | 1110 | 1260 | 2195 | 2375 | 2450 | 2635 | 955 |
|  | 11-7/8" | 970 | 1275 | 1160 | 1485 | 2330 | 2525 | 2595 | 2830 |  |
|  | 14" | 970 | 1395 | 1200 | 1680 | 2455 | 2665 | 2725 | 3005 |  |
|  | $16^{\prime \prime}$ | 970 | 1510 | 1240 | 1870 | 2570 | 2795 | 2850 | 3175 |  |
| LPI 32Plus | 9-1/2" | 970 | 1140 | 1110 | 1260 | 2195 | 2375 | 2450 | 2635 | 1180 |
|  | 11-7/8" | 970 | 1275 | 1160 | 1485 | 2330 | 2525 | 2595 | 2830 |  |
|  | 14" | 970 | 1395 | 1200 | 1680 | 2455 | 2665 | 2725 | 3005 |  |
|  | 16" | 970 | 1510 | 1240 | 1870 | 2570 | 2795 | 2850 | 3175 |  |
| LPI 36 | 11-7/8" | 1025 | 1500 | 1290 | 1615 | 2500 | 3105 | 2835 | 3470 | 1180 |
|  | 14 " | 1025 | 1515 | 1325 | 1830 | 2500 | 3205 | 2835 | 3565 |  |
|  | 16" | 1025 | 1525 | 1360 | 2020 | 2500 | 3305 | 2835 | 3655 |  |
| LPI 42Plus | 9-1/2" | 1185 | 1340 | 1305 | 1340 | 2900 | 3095 | 2940 | 3195 | 1705 |
|  | 11-7/8" | 1245 | 1510 | 1595 | 1625 | 3025 | 3340 | 3120 | 3515 |  |
|  | $14 "$ | 1300 | 1660 | 1595 | 1875 | 3140 | 3565 | 3280 | 3805 |  |
|  | $16^{\prime \prime}$ | 1350 | 1800 | 1595 | 2115 | 3245 | 3775 | 3435 | 4080 |  |
| LPI 52Plus | 11-7/8" | 1370 | 1820 | 1690 | 2055 | 3420 | 4000 | 3635 | 4210 | 1995 |
|  | 14" | 1385 | 1970 | 1845 | 2330 | 3435 | 4260 | 3745 | 4540 |  |
|  | 16" | 1400 | 2110 | 1985 | 2585 | 3450 | 4505 | 3850 | 4855 |  |
| LPI 56 | 11-7/8" | 1145 | 1660 | 1515 | 2055 | 3130 | 3860 | 3670 | 4060 | 1870 |
|  | $14 "$ | 1145 | 1755 | 1535 | 2330 | 3130 | 4055 | 3670 | 4300 |  |
|  | 16" | 1145 | 1845 | 1555 | 2585 | 3130 | 4245 | 3670 | 4525 |  |

## NOTES

1. End and Interior Reaction Capacity shall be limited by the Flange Bearing Capacity or the bearing capacity of the support material, whichever is less.
2. The Flange Bearing Capacity, per inch of bearing length, is based on the allowable compression perpendicular-to-grain of the I-Joist flange, accounting for eased edges.
3. To account for edge easing when determining the bearing capacity of the support material, subtract $0.25^{\prime \prime}$ from the flange width for the LPI 18, LPI 20Plus, LPI 32Plus, LPI 42Plus \& LPI 52Plus, and subtract 0.10" from the flange width for the LPI 36 \& LPI 56.
4. Reaction Capacity is for normal load duration and shall be adjusted according to code. Flange Bearing Capacity and the bearing capacity of any wood support shall not be adjusted for load duration.
5. Reaction Capacity and Flange Bearing Capacity may be increased over that tabulated for the minimum bearing length. Linear interpolation of the Reaction Capacity between the minimum and maximum bearing length is permitted. Bearing lengths longer than the maximum do not further increase Reaction Capacity. Flange Bearing Capacity and that of a wood support will increase with additional bearing length.
6. See page 28 for information on web stiffener sizes and nailing

## EXAMPLE:

Determine the stiffened end reaction capacity for a 14 " LPI 32Plus with 2" of bearing for a non-snow roof load and supported on an SPF wall plate (425 psi).

1. Determine End Reaction (ER) w/Stiffeners $E R=1395+(1660-1395)^{*}\left(2^{\prime \prime}-1.5^{\prime \prime}\right) /\left(4^{\prime \prime}-1.5^{\prime \prime}\right)=1448 \mathrm{lbs}$
2. Adjust for load duration

Adjusted ER $=1448$ * $1.25=1810 \mathrm{lbs}$
3. Determine Flange Bearing Capacity (FBC): FBC $=1180 \mathrm{lb} / \mathrm{in}^{*} 2^{\prime \prime}=2360 \mathrm{lbs}$
4. Determine wall Plate Bearing Capacity (PBC): PBC $=425$ psi $^{*}\left(2.5^{\prime \prime}-0.25^{\prime \prime}\right)^{*} 2^{\prime \prime}=1912 \mathrm{lbs}$ 5. Final End Reaction Capacity w/Stiffeners $=1810 \mathrm{lbs}$

PROFILE DETAILS


## TO USE:

1. Select the Simple Span or Continuous Span table, as required
2. Find a span that meets or exceeds the required clear span
3. Read the corresponding joist series, depth and spacing.

CAUTION: For floor systems that require both simple span and continuous span joists, it is a good idea to check both before selecting a joist. Some conditions are controlled by continuous span rather than simple span.


| SIMPLE SPAN |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Series | Depth | L/480 |  |  |  | L/360 |  |  |  |
|  |  | 12" oc | 16" oc | 19.2" oc | 24" oc | 12" oc | 16" oc | 19.2" oc | 24" oc |
| LPI 18 | 9-1/2" | 16'-6" | 15'-1" | $14^{\prime}$ '-3" | 13'-4" | 18'-3" | 16'-8" | $15^{\prime}-3{ }^{\prime \prime}$ | 13'-7" |
|  | 11-7/8" | 19'-9" | 18'-1" | 17'-1" | 15'-7" | 21'-10" | 19'-1" | 17'-5" | 15'-7" |
|  | 14" | 22'-6" | 20'-7" | 19'-1" | 17'-1" | 24'-3" | 21'-0" | 19'-1" | 17'-1" |
| LPI 20Plus | 9-1/2" | 17'-9" | $16^{\prime}-2{ }^{\prime \prime}$ | 15'-3" | 14'-3" | 19'-7" | 17'-11" | 16'-7" | 14'-10" |
|  | 11-7/8" | 21'-2" | 19'-4" | 18'-3" | 17'-0" | 23'-5" | 21-1" | 19'-3" | 17'-2" |
|  | $14{ }^{\prime \prime}$ | 24'-1" | 22-0" | 20'-9" | 18'-7" | 26'-4" | 22'-10" | 20'-10" | 18'-7" |
|  | $16^{\prime \prime}$ | 26'-9" | 24'-5" | 22'-4" | 19'-7" | 28'-3" | 24'-5" | 22'-4" | 19'-7" |
| LPI 32Plus | 9-1/2" | 18'-9" | 17 '-0" | $16^{\prime}-0{ }^{\prime \prime}$ | 14'-9" | 20'-10" | 18'-11" | 17'-10" | 16'-6" |
|  | 11-7/8" | 22'-3" | 20'-2" | 19'-0" | 17'-7" | 24'-9" | 22'-6" | 21'-2" | 19'-2" |
|  | $14 "$ | 25'-2" | 22'-10" | 21'-6" | 19'-6" | 28'-0" | 25'-5" | $23^{\prime}-7{ }^{\prime \prime}$ | 19'-6" |
|  | $16 "$ | 27'-10" | 25'-3" | 23'-9" | 19'-7" | 30'-11" | 27-10" | 24'-7" | 19'-7" |
| LPI 36 | 11-7/8" | 23'-1" | 21'-1" | 19'-11" | 18'-6" | 25'-5" | $23^{\prime \prime}-{ }^{\prime \prime}$ | 22'-1" | 20'-6" |
|  | 14" | 26'-2" | 23'-10" | 22'-6" | 20'-9" | 28-11" | 26'-5" | 24'-11" | 20'-9" |
|  | $16^{\prime \prime}$ | 28'-10" | 26'-4" | $24^{\prime}-10{ }^{\prime \prime}$ | 20'-10" | 31'-11" | 29'-2" | 26'-2" | 20'-10" |
| LPI 42Plus | 9-1/2" | 20'-10" | 19'-0" | 17'-11" | 16'-8" | 23'-1" | 21'-1" | 19'-11" | 18'-6" |
|  | 11-7/8" | 24'-11" | 22'-8" | 21-4" | 19'-10" | 27'-6" | 25'-1" | 23'-8" | 22'-0" |
|  | $14 "$ | 28'-3" | 25'-9" | 24'-3" | 22'-6" | 31'-3" | 28'-6" | 26'-10" | 25'-0" |
|  | $16^{\prime \prime}$ | 31'-4" | 28'-6" | 26'-10" | 25'-0" | 34'-7" | 31'-7" | 29'-9" | 27'-2" |
| LPI 52Plus | 11-7/8" | 25'-9" | 23'-5" | 22'-1" | 20'-7" | 28'-5" | 25-11" | 24'-6" | 22'-10" |
|  | 14" | 29'-2" | 26'-7" | 25'-0" | 23'-4" | 32'-3" | 29'-5" | 27'-9" | 25'-10" |
|  | 16" | 32'-3" | 29'-4" | 27'-8" | 25'-9" | 35'-7" | 32'-6" | 30'-8" | 28'-7" |
| LPI 56 | 11-7/8" | 26'-6" | 24'-1" | 22'-8" | 21'-1" | 29'-3" | 26'-8" | 25'-2" | 23'-4" |
|  | 14" | 29'-11" | 27'-3" | 25'-8" | 23'-4" | 33'-1" | 30'-2" | 28'-5" | 23'-4" |
|  | $16^{\prime \prime}$ | $33^{\prime}-1{ }^{1 \prime}$ | 30'-1" | 28'-4" | 23'-5" | $36^{\prime}-7{ }^{\prime \prime}$ | 33'-4" | 29'-4" | 23'-5" |



## DESIGN ASSUMPTIONS:

1. The spans listed are the clear distance between supports. Continuous spans are based on the longest span. The shortest span shall not be less than $50 \%$ of the longest span.
2. The spans are based on uniform floor loads only as listed at the top of the page The dead load is increased to 12 psf for the LPI 42Plus, LPI 52Plus and LPI 56.
3. These tables reflect the additional stiffness provided by 48/24 APA RATED SHEATHING or 24 oc APA RATED STURD-I-FLOOR, or equal, glued and nailed to the top flange.
4. Live Load deflection is limited to $L / 480$ or $L / 360$ for simple spans as listed, and $L / 480$ only for continuous spans.
5. Total Load deflection is limited to L/240
6. The spans are based on an end bearing length of at least $1-3 / 4$ " and an interior bearing length of at least 3-1/2," and are limited to the bearing capacity for an SPF wall plate ( $\mathrm{F}_{\mathrm{c} \perp}=425 \mathrm{psi}$ ).

## ADDITIONAL NOTES:

1. Web stiffeners are not required for the Simple Span tables. Web stiffeners are not required at the end bearings for the Continuous Span tables. Web stiffeners at interior supports are only required where listed in the "With Web Stiffeners" section of each table. A "-" indicates no increase in span with web stiffeners.
2. Web fillers are required for I-Joists seated in hangers that do not laterally support the top flange.
3. $L / 360$ represents the maximum deflection allowed per code and may not provide suitable floor performance. L/480 or better is recommended for most applications.
4. These spans are not evaluated for vibration.
5. Though not required for the spans above, bridging, blocking, bottom-flange bracing or a direct-applied gypsum ceiling can improve the feel of a floor.
6. For conditions not shown, use the Uniform Floor Load (PLF) tables, LP's design software or contact your LP ${ }^{\oplus}$ SolidStart ${ }^{\oplus}$ Engineered Wood Products distributor for assistance.

## TO USE:

1. Select the Simple Span or Continuous Span table, as required
2. Find a span that meets or exceeds the required clear span
3. Read the corresponding joist series, depth and spacing.

CAUTION: For floor systems that require both simple span and continuous span joists, it is a good idea to check both before selecting a joist. Some conditions are controlled by continuous span rather than simple span.


| SIMPLE SPAN |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Series | Depth | L/480 |  |  |  | L/360 |  |  |  |
|  |  | 12" oc | 16" oc | 19.2" oc | 24" oc | 12" oc | 16" oc | 19.2" oc | 24" oc |
| LPI 18 | 9-1/2" | 16'-6" | 15'-1" | 14'-3" | 12'-11" | 18'-3" | 15'-11" | 14'-6" | 12'-11" |
|  | 11-7/8" | 19'-9" | 18'-1" | $16^{\prime}-7{ }^{\prime \prime}$ | 14'-10" | 21-11 | 18'-3" | $16^{\prime}-7{ }^{\prime \prime}$ | 14'-10" |
|  | 14" | 22'-6" | 20'-0" | 18'-3" | 15'-10" | 23'-1" | 20'-0" | 18'-3" | 15'-10" |
| LPI 20Plus | 9-1/2" | 17'-9" | 16'-2" | $15^{\prime}-3$ " | 14'-2" | 19'-7" | 17'-4" | 15'-10" | 14'-2" |
|  | 11-7/8" | 21-2" | 19'-4" | 18'-3" | 16'-4" | 23'-2" | 20'-1" | 18'-4" | 16'-4" |
|  | $14 "$ | 24'-1" | 21-9" | 19'-10" | 17'-9" | 25'-2" | 21-9" | 19'-10" | 17'-9" |
|  | $16^{\prime \prime}$ | 26-9" | 23'-4" | 21'-3" | 17'-10" | 26'-11" | 23'-4" | 21-3" | 17'-10" |
| LPI 32Plus | 9-1/2" | 18'-9" | 17'-0" | $16^{\prime}-0 \mid$ | 14'-9" | 20'-10" | 18'-11" | 17'-10" | $16^{\prime}-1{ }^{\prime \prime}$ |
|  | 11-7/8" | 22'-3" | 20'-2" | 19'-0" | 17'-7" | 24'-9" | 22'-5" | 20'-6" | 17'-8" |
|  | $14 "$ | 25'-2" | 22'-10" | 21'-6" | 17'-9" | 28'-0" | 24'-8" | $22^{\prime}-3{ }^{\prime \prime}$ | 17'-9" |
|  | 16" | 27'-10" | 25'-3" | $22^{\prime}-4 "$ | 17'-10" | 30'-8" | 26'-7" | 22'-4" | 17'-10" |
| LPI 36 | 11-7/8" | 23'-1" | 21'-1" | 19'-11" | 18'-6" | 25'-5" | 23'-4" | 22'-1" | 18'-10" |
|  | $14 "$ | 26'-2" | 23 '-10" | 22'-6" | 18'-10" | 28'-11" | 26'-5" | $23^{\prime \prime}-8{ }^{\prime \prime}$ | 18'-10" |
|  | $16^{\prime \prime}$ | 28'-10" | 26'-4" | 23'-9" | $18^{\prime}-11^{\prime \prime}$ | 31'-11" | 28'-7" | 23'-9" | 18'-11" |
| LPI 42Plus | 9-1/2" | 20'-10" | 19'-0" | 17'-11" | 16'-8" | 23'-1" | 21-1" | 19'-11" | 18'-6" |
|  | 11-7/8" | 24'-11" | 22'-8" | 21'-4" | 19'-10" | 27'-6" | $25^{\prime}-1{ }^{\prime \prime}$ | $23^{\prime}-8{ }^{\prime \prime}$ | 22'-0" |
|  | $14 "$ | 28'-3" | 25-9" | 24'-3" | 22'-6" | 31'-3" | 28'-6" | 26'-10" | $23^{\prime}-10{ }^{\prime \prime}$ |
|  | $16^{\prime \prime}$ | 31-4" | 28'-6" | 26'-10" | $24^{\prime}-8^{\prime \prime}$ | 34'-7" | 31'-7" | 29'-7" | 24'-8" |
| LPI 52Plus | 11-7/8" | 25'-9" | 23'-5" | 22'-1" | 20'-7" | 28'-5" | 25'-11" | 24'-6" | 22'-10" |
|  | 14" | 29'-2" | 26'-7" | $25^{\prime}-0 \mid$ | 23'-4" | 32'-3" | 29'-5" | 27'-9" | 25'-8" |
|  | $16^{\prime \prime}$ | 32'-3" | 29'-4" | 27'-8" | 25'-9" | 35'-7" | 32'-6" | 30'-8" | 26'-2" |
| LPI 56 | 11-7/8" | 26'-6" | 24'-1" | $22^{\prime}-8{ }^{\prime \prime}$ | 21'-1" | 29'-3" | 26'-8" | 25'-2" | 21'-2" |
|  | 14" | 29'-11" | 27'-3" | 25'-8" | 21'-2" | 33'-1" | 30'-2" | 26'-7" | 21'-2" |
|  | $16^{\prime \prime}$ | 33'-1" | 30'-1" | $26^{\prime}-7{ }^{\prime \prime}$ | 21'-3" | $36^{\prime}-7{ }^{\prime \prime}$ | 32-0" | 26'-7" | 21'-3" |



## DESIGN ASSUMPTIONS:

1. The spans listed are the clear distance between supports. Continuous spans are based on the longest span. The shortest span shall not be less than $50 \%$ of the longest span.
2. The spans are based on uniform floor loads only as listed at the top of the page.
3. These tables reflect the additional stiffness provided by 48/24 APA RATED SHEATHING or 24 oc APA RATED STURD-I-FLOOR, or equal, glued and nailed to the top flange.
4. Live Load deflection is limited to $L / 480$ or $L / 360$ for simple spans as listed, and $L / 480$ only for continuous spans.
5. Total Load deflection is limited to L/240.
6. The spans are based on an end bearing length of at least $1-3 / 4^{\prime \prime}$ and an interior bearing length of at least 3-1/2", and are limited to the bearing capacity for an SPF wall plate ( $\mathrm{F}_{\mathrm{c} \perp}=425 \mathrm{psi}$ ).

## ADDITIONAL NOTES:

1. Web stiffeners are not required for the Simple Span tables. Web stiffeners are not required at the end bearings for the Continuous Span tables. Web stiffeners at interior supports are only required where listed in the "With Web Stiffeners" section of each table. A "-" indicates no increase in span with web stiffeners.
2. Web fillers are required for I-Joists seated in hangers that do not laterally support the top flange.
3. $L / 360$ represents the maximum deflection allowed per code and may not provide suitable floor performance. L/480 or better is recommended for most applications.
4. These spans are not evaluated for vibration.
5. Though not required for the spans above, bridging, blocking, bottom-flange bracing or a direct-applied gypsum ceiling can improve the feel of a floor.
6. For conditions not shown, use the Uniform Floor Load (PLF) tables, LP's design software or contact your LP ${ }^{\ominus}$ SolidStart ${ }^{\oplus}$ Engineered Wood Products distributor for assistance.

## TO USE:

1. Select the Simple Span or Continuous Span table, as required
2. Find a span that meets or exceeds the required clear span
3. Read the corresponding joist series, depth and spacing.

CAUTION: For floor systems that require both simple span and continuous span joists, it is a good idea to check both before selecting a joist. Some conditions are controlled by continuous span rather than simple span


| SIMPLE SPAN |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Series | Depth | L/480 |  |  |  | L/360 |  |  |  |
|  |  | 12" oc | 16" oc | 19.2" oc | 24" oc | 12" oc | 16" oc | 19.2" oc | 24" oc |
| LPI 18 | 9-1/2" | 16'-6" | 14'-7" | $13^{\prime}-4$ " | 11'-11" | 16'-11" | 14'-7" | $13^{\prime}-4$ " | 11'-11" |
|  | 11-7/8" | 19'-4" | 16'-9" | $15^{\prime}-3 "$ | 13'-4" | 19'-4" | 16'-9" | 15'-3" | $13^{\prime}-4^{\prime \prime}$ |
|  | 14" | 21'-3" | 18'-4" | 16'-9" | 13'-4" | 21'-3" | 18'-4" | 16'-9" | 13'-4" |
| LPI 20Plus | 9-1/2" | 17'-9" | 15'-11" | 14'-6" | 13'-0" | 18'-5" | 15'-11" | 14'-6" | 13'-0" |
|  | 11-7/8" | 21'-2" | 18'-5" | 16'-10" | 14'-11" | 21'-4" | 18'-5" | 16'-10" | 14'-11" |
|  | $14 "$ | 23'-1" | 20'-0" | 18'-3" | 14'-11" | 23'-1" | 20'-0" | 18'-3" | 14'-11" |
|  | $16^{\prime \prime}$ | 24-9" | 21'-5" | 18'-10" | 15'-0" | 24'-9" | 21'-5" | 18'-10" | 15'-0" |
| LPI 32Plus | 9-1/2" | 18'-9" | 17'-0" | $16^{\prime}-0{ }^{\prime \prime}$ | 14'-9" | 20'-3" | 18'-1" | 16'-6" | 14'-9" |
|  | 11-7/8" | 22'-3" | 20'-2" | 18'-8" | 14'-11" | 23'-10" | 20'-8" | 18'-8" | 14'-11" |
|  | $14 "$ | 25'-2" | 22'-7" | 18'-9" | 14'-11" | 26'-2" | 22'-7" | 18'9" | 14'-11" |
|  | $16^{\prime \prime}$ | 27'-10" | 22'-8" | 18'-10" | 15'-0" | 28'-3" | 22'-8" | 18'-10" | 15'-0" |
| LPI 36 | 11-7/8" | 23'-1" | 21'-1" | 19'-11" | 15'-10" | 24'-10" | 22'-8" | 19'-11" | 15'-10" |
|  | 14" | 26'-2" | 23'-10" | 19'-11" | 15'-11" | 28'-2" | 24'-0" | 19'-11" | 15'-11" |
|  | $16^{\prime \prime}$ | 28'-10" | 24'-1" | 20'-0" | 16'-0" | 31'-0" | 24'-1" | 20'-0" | $16^{\prime}-0^{\prime \prime}$ |
| LPI 42Plus | 9-1/2" | 20'-10" | 19'-0" | 17'-11" | 16'-8" | 22'-5" | 20'6" | 19'-4" | 18'-0" |
|  | 11-7/8" | 24'-11" | 22'-8" | 21-4" | 19'-4" | 26'-9" | 24'-5" | 23'-0" | 19'-4" |
|  | 14" | 28'-3" | 25'-9" | 24'-3" | 20'-2" | 30'-5" | 27'-8" | 25'-3" | 20'-2" |
|  | $16^{\prime \prime}$ | 31'-4" | 28'-6" | 26'-1" | 20'-10" | 33'-8" | 29'-10" | $26^{\prime}-1{ }^{\prime \prime}$ | 20'-10" |
| LPI 52Plus | 11-7/8" | 25-9" | 23'-5" | 22'-1" | 20'-7" | 27'-8" | 25'-3" | 23'-10" | 21'-3" |
|  |  | 29'-2" | 26'-7" | 25'-0" | 21-8" | 31'-4" | 28'-7" | 26'-11" | 21'-8" |
|  | 16" | 32-3" | 29'-4" | 27'-8" | 22-1" | 34'-8" | 31'-7" | 27'-9" | 22'-1" |
| LPI 56 | 11-7/8" | 26-6" | 24'-1" | 22'-5" | 17'-10" | 28'-6" | 25'-11" | 22'-5" | 17'-10" |
|  | $14 "$ | 29'-11" | 27'-0" | 22'-5" | 17'-11" | 32'-2" | 27'-0" | 22'-5" | 17'-11" |
|  | $16^{\prime \prime}$ | $33^{\prime}-1{ }^{11}$ | 27'-0" | $22^{\prime}-6{ }^{\prime \prime}$ | 17'-11" | 35'-7" | 27'-0" | $22^{\prime}-6{ }^{\prime \prime}$ | $17^{\prime}-11^{\prime \prime}$ |

## CONTINUOUS SPAN

| Series | Depth | L/480; No Web Stiffeners |  |  |  | L/480; With Web Stiffeners |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Series |  | 12 oc | 16 oc | 19.2" oc | 24" oc | 12" oc | 16 " oc | 19.2" oc | 24" oc |
| LPI 18 | 9-1/2" | 16'-10" | 14'-6" | 13'-3" | 11'-10" | - | - | - | - |
|  | 11-7/8" | 19'-3" | 16'-8" | 15'-2" | 12'-7" | - | - | - | $13^{\prime}-7{ }^{\prime \prime}$ |
|  | 14" | 21'-2" | $18^{\prime}-3$ " | 16'-8" | 13'-3" | - | - | - | 14'-5" |
| LPI 20Plus | 9-1/2" | 18'-4" | 15'-10" | 14'-5" | 12'-11" | - | - | - | - |
|  | 11-7/8" | 21'-3" | 18'-4" | 16'-9" | 14'-0" | - | - | - | 14'-11" |
|  | 14" | 23'-0" | 19'-11" | 18'-2" | 14'-9" | - | - | - | 16'-1" |
|  | 16" | 24'-8" | 21'-4" | 19'-5" | 15'-6" | - | - | - | 16'-10" |
| LPI 32Plus | 9-1/2" | 20'-4" | 18'-0" | 16'-5" | 13'-2' | - | - | - | 14'-3" |
|  | 11-7/8" | 23'-9" | 20'-7" | 17'-7" | 14'-0" | - | - | 18'-9" | 15'-2" |
|  | 14" | 26'-1' | 22'-4" | 18'-7" | 14'-9" | - | 22'-7" | 20'-2" | 16'-1" |
|  | 16" | 28'-2" | 23'-5" | 19'-5" | 15'-6" | - | 24'-4" | 21'-2" | 16'-10" |
| LPI 36 | 11-7/8" | 25'-2" | 22'-9" | 18'-11" | 15'-1" | - | 22'-11" | 21-7" | 18'-5" |
|  | 14" | 28'-6" | 22'-9" | 18'-11" | 15'-1" | - | 26'-0" | 23'-3" | 18'-6" |
|  | 16" | 30'-5" | 22'-9" | 18'-11" | $15^{\prime}-1{ }^{\prime \prime}$ | 31'-6" | 28'-0" | 23'-4" | 18'-7" |
| LPI 42Plus | 9-1/2" | 22'-9" | 20'-8" | 19'-6" | 16'-3" | - | - | - | - |
|  | 11-7/8" | 27'-1" | 24'-8" | 22'-11" | 18'-3" | - | - | - | 19'-9" |
|  | 14" | 30'-10" | 27'-7" | 23'-10" | 19'-0" | - | - | 25'-2" | 21'-7" |
|  | 16" | 34'-2" | 29'-7" | 24'-8" | 19'-8" | - | 29'-9" | 27'-1' | 22'-11" |
| LPI 52Plus | 11-7/8" | 28'-1" | 25'-7" | 24'-1" | 20'-9" | - | - | - | 22'-5" |
|  | 14" | 31'-10" | 28'-11" | 26'-1" | 20'-10" | - | - | 27'-3" | 24'-10" |
|  | 16" | 35'-2" | 31'-6" | 26'-2" | 20'-11" | - | 32'-0" | 29'-11" | 25'-9" |
| LPI 56 | 11-7/8" | 28'-10" | 26'-3' | 23'-9" | 18'-11" | - | - | 24'-8" | 20'-9" |
|  | 14" | $32^{\prime}-8{ }^{\prime \prime}$ | 28'-7" | 23'-9" | 18'-11" | - | 29'-8" | 26'-1" | 20'-10" |
|  | 16" | $36^{\prime}-1{ }^{\prime \prime}$ | 28'-7" | 23'-9" | 18'-11" | - | 31'-5" | $26^{\prime}-2^{\prime \prime}$ | 20'-10" |

## DESIGN ASSUMPTIONS:

1. The spans listed are the clear distance between supports. Continuous spans are based on the longest span. The shortest span shall not be less than $50 \%$ of the longest span
2. The spans are based on uniform floor loads only as listed at the top of the page.
3. These tables reflect the additional stiffness provided by $48 / 24$ APA RATED SHEATHING or 24 oc APA RATED STURD-I-FLOOR, or equal, glued and nailed to the top flange.
4. Live Load deflection is limited to $L / 480$ or $L / 360$ for simple spans as listed, and $L / 480$ only for continuous spans.
5. Total Load deflection is limited to L/240.
6. The spans are based on an end bearing length of at least $1-3 / 4^{\prime \prime}$ and an interior bearing length of at least 3-1/2", and are limited to the bearing capacity for an SPF wall plate ( $\mathrm{F}_{\mathrm{c} \perp}=425 \mathrm{psi}$ ).

## ADDITIONAL NOTES:

1. Web stiffeners are not required for the Simple Span tables. Web stiffeners are not required at the end bearings for the Continuous Span tables. Web stiffeners at interior supports are only required where listed in the "With Web Stiffeners" section of each table. A "-" indicates no increase in span with web stiffeners.
2. Web fillers are required for I-Joists seated in hangers that do not laterally support the top flange.
3. $L / 360$ represents the maximum deflection allowed per code and may not provide suitable floor performance. L/480 or better is recommended for most applications.
4. These spans are not evaluated for vibration.
5. Though not required for the spans above, bridging, blocking, bottom-flange bracing or a direct-applied gypsum ceiling can improve the feel of a floor.
6. For conditions not shown, use the Uniform Floor Load (PLF) tables, LP's design software or contact your LP ${ }^{\oplus}$ SolidStart ${ }^{\oplus}$ Engineered Wood Products distributor for assistance.

## TO USE:

1. Select the span required
2. Compare the design total load to the Total Load column.
3. Compare the design live load to the appropriate Live Load column.
4. Select a product that exceeds both the design total and live loads.

## EXAMPLE:

Select an I-Joist for a $17^{\prime \prime}-6$ " clear span supporting 40 psf Live Load and 20 psf Dead Load, spaced 16 " oc, at an L/480 deflection limit.

1. Select the row corresponding to an 18' span.
2. Design Total Load $=(40+20) *(16 / 12)=80$ plf

Design Live Load $=40$ * $(16 / 12)=54 \mathrm{plf}$
3. Select the first joist to exceed both Total Load and L/480:

The $9-1 / 2^{\prime \prime}$ LPI 42Plus supports 108 plf Total Load and 54 plf Live Load at L/480

| Span | 9-1/2" LPI 18 |  |  | 9-1/2" LPI 20 Plus |  |  | 9-1/2" LPI 32Plus |  |  | 9-1/2" LPI 42Plus |  |  | Span |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Live Load |  | Total Load | Live Load |  | $\begin{aligned} & \text { Total } \\ & \text { Load } \end{aligned}$ | Live Load |  | $\begin{aligned} & \text { Total } \\ & \text { Load } \end{aligned}$ | Live Load |  | $\begin{aligned} & \text { Total } \\ & \text { Load } \end{aligned}$ |  |
|  | L/480 | L/360 |  | L/480 | L/360 |  | L/480 | L/360 |  | L/480 | L/360 |  |  |
| 8' |  |  | 190 |  |  | 211 |  |  | 211 |  |  | 260 | 8' |
| $9^{\prime}$ |  |  | 170 |  |  | 188 |  |  | 188 |  |  | 232 | $9^{\prime}$ |
| 10' | 128 |  | 153 | 160 |  | 170 |  |  | 170 |  |  | 209 | 10' |
| 11' | 99 | 132 | 139 | 125 |  | 155 | 138 |  | 155 |  |  | 191 | 11' |
| 12' | 78 | 104 | 126 | 99 | 132 | 142 | 112 |  | 142 | 161 |  | 175 | 12' |
| 13' | 62 | 83 | 108 | 79 | 106 | 128 | 91 | 122 | 132 | 130 |  | 162 | 13' |
| 14' | 51 | 68 | 93 | 65 | 86 | 111 | 76 | 101 | 122 | 107 | 143 | 150 | 14' |
| 15' | 42 | 56 | 81 | 53 | 71 | 97 | 63 | 84 | 114 | 89 | 118 | 140 | 15' |
| 16' | 35 | 46 | 70 | 44 | 59 | 85 | 53 | 71 | 107 | 74 | 99 | 132 | 16' |
| 17' | 29 | 39 | 59 | 37 | 50 | 75 | 45 | 60 | 91 | 63 | 84 | 124 | 17' |
| 18' | 25 | 33 | 50 | 32 | 42 | 64 | 39 | 52 | 78 | 54 | 72 | 108 | 18' |
| 19' | 21 | 28 | 42 | 27 | 36 | 55 | 33 | 45 | 67 | 46 | 61 | 92 | 19' |
| 20' | 18 | 24 | 36 | 23 | 31 | 47 | 29 | 39 | 58 | 40 | 53 | 80 | 20' |
| 21' | 16 | 21 | 32 | 20 | 27 | 41 | 25 | 34 | 51 | 35 | 46 | 70 | 21' |
| 22' | - | - | - | - | - | - | - | - | - | 30 | 40 | 61 | 22' |
| 23' | - | - | - | - | - | - | - | - | - | 27 | 36 | 54 | $23^{\prime}$ |
| 24' | - | - | - | - | - | - | - | - | - | - | - | - | 24' |
| 25' | - | - | - | - | - | - | - | - | - | - | - | - | $25^{\prime}$ |
| 26' | - | - | - | - | - | - | - | - | - | - | - | - | 26' |
| 27' | - | - | - | - | - | - | - | - | - | - | - | - | $27^{\prime}$ |
| 28' | - | - | - | - | - | - | - | - | - | - | - | - | 28' |


| Span | 11-7/8" LPI 18 |  |  | 11-7/8" LPI 20Plus |  |  | 11-7/8" LPI 32Plus |  |  | 11-7/8" LPI 36 |  |  | 11-7/8" LPI 42Plus |  |  | 11-7/8" LPI 52Plus |  |  | 11-7/8" LPI 56 |  |  | Span |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Live Load |  | Total Load | Live Load |  | Total Load | Live Load |  | Total Load | Live Load |  | Total Load | Live Load |  | Total <br> Load | Live Load |  | Total Load | Live Load |  | Total Load |  |
|  | L/480 | L/360 |  | L/480 | L/360 |  | L/480 | L/360 |  | L/480 | L/360 |  | L/480 | L/360 |  | L/480 | L/360 |  | L/480 | L/360 |  |  |
| 8' |  |  | 202 |  |  | 224 |  |  | 224 |  |  | 241 |  |  | 291 |  |  | 329 |  |  | 285 | 8' |
| $9{ }^{\prime}$ |  |  | 180 |  |  | 200 |  |  | 200 |  |  | 215 |  |  | 260 |  |  | 294 |  |  | 254 | $9{ }^{\prime}$ |
| 10' |  |  | 162 |  |  | 181 |  |  | 181 |  |  | 194 |  |  | 235 |  |  | 265 |  |  | 229 | 10' |
| 11' |  |  | 148 |  |  | 165 |  |  | 165 |  |  | 177 |  |  | 214 |  |  | 242 |  |  | 209 | 11' |
| 12' | 131 |  | 136 |  |  | 151 |  |  | 151 |  |  | 162 |  |  | 196 |  |  | 222 |  |  | 192 | 12' |
| 13' | 105 |  | 126 | 131 |  | 140 |  |  | 140 |  |  | 150 |  |  | 182 |  |  | 205 |  |  | 177 | 13' |
| 14' | 86 | 115 | 117 | 107 |  | 130 | 119 |  | 130 | 139 |  | 139 |  |  | 169 |  |  | 191 |  |  | 165 | 14' |
| 15' | 71 | 95 | 107 | 89 | 118 | 121 | 100 |  | 121 | 116 |  | 130 | 145 |  | 158 | 162 |  | 178 |  |  | 154 | 15' |
| 16' | 59 | 79 | 94 | 74 | 99 | 114 | 85 | 113 | 114 | 97 |  | 122 | 122 |  | 148 | 136 |  | 167 |  |  | 145 | 16' |
| 17' | 50 | 67 | 83 | 63 | 84 | 101 | 72 | 97 | 107 | 83 | 110 | 115 | 103 | 138 | 139 | 115 | 154 | 158 | 124 |  | 136 | 17' |
| 18' | 42 | 57 | 74 | 53 | 71 | 90 | 62 | 83 | 101 | 70 | 94 | 109 | 88 | 118 | 132 | 98 | 131 | 149 | 106 |  | 129 | 18' |
| 19' | 36 | 48 | 67 | 46 | 61 | 81 | 54 | 72 | 96 | 61 | 81 | 103 | 76 | 102 | 125 | 85 | 113 | 141 | 92 |  | 122 | 19' |
| 20' | 31 | 42 | 60 | 40 | 53 | 73 | 47 | 63 | 91 | 52 | 70 | 98 | 66 | 88 | 119 | 73 | 98 | 134 | 80 | 106 | 116 | $20^{\prime}$ |
| 21 | 27 | 36 | 55 | 34 | 46 | 66 | 41 | 55 | 82 | 46 | 61 | 92 | 58 | 77 | 113 | 64 | 85 | 128 | 70 | 93 | 111 | 21 |
| 22' | 24 | 32 | 48 | 30 | 40 | 60 | 36 | 48 | 73 | 40 | 54 | 81 | 51 | 68 | 102 | 56 | 75 | 113 | 61 | 82 | 106 | $22^{\prime}$ |
| 23' | 21 | 28 | 42 | 26 | 35 | 53 | 32 | 43 | 64 | 35 | 47 | 71 | 45 | 60 | 90 | 49 | 66 | 99 | 54 | 72 | 101 | 23' |
| 24' | 18 | 25 | 37 | 23 | 31 | 47 | 28 | 38 | 57 | 31 | 42 | 63 | 39 | 53 | 79 | 44 | 58 | 88 | 48 | 64 | 96 | $24^{\prime}$ |
| $25^{\prime}$ | 16 | 22 | 33 | 21 | 28 | 42 | 25 | 34 | 51 | 28 | 37 | 56 | 35 | 47 | 71 | 39 | 52 | 78 | 43 | 57 | 86 | $25^{\prime}$ |
| 26' | 14 | 19 | 29 | 18 | 25 | 37 | 22 | 30 | 45 | - | - | - | 31 | 42 | 63 | 35 | 46 | 70 | 38 | 51 | 76 | 26' |
| 27' | - | - | - | - | - | - | - | - | - | - | - | - | 28 | 38 | 57 | 31 | 42 | 63 | 34 | 46 | 69 | $27^{\prime}$ |
| 28' | - | - | - | - | - | - | - | - | - | - | - | - | 25 | 34 | 51 | 28 | 37 | 56 | 31 | 41 | 62 | $28^{\prime}$ |

## DESIGN ASSUMPTIONS:

1. Span is the clear distance between supports and is valid for simple or continuous span applications. Continuous spans are based on the longest span. The shortest span shal not be less than $50 \%$ of the longest span.
2. The values in the tables are for uniform loads only.
3. Total Load is for normal ( $100 \%$ ) duration
4. These tables do not reflect any additional stiffness provided by the floor sheathing.
5. Live Load deflection is limited to $L / 360$ or $L / 480$ as noted in the table.
6. Total Load deflection is limited to $L / 240$. Long term deflection (creep) has not been considered.
7. These tables assume full lateral support of the compression flange (maximum unbraced length of 24.)
8. These tables are based on an end bearing length of at least $1-3 / 4$ " and an interior bearing length of at least $3-1 / 2$," and are limited to the bearing capacity for an SPF wall plate ( $\left.\mathrm{F}_{\mathrm{c} \perp}=425 \mathrm{psi}\right)$.

## ADDITIONAL NOTES

1. The allowable loads represent the capacity of the joist in pounds per lineal foot (plf) of length.
2. The designer shall check both the Total Load and the appropriate Live Load column.
3. To design a double l-Joist, the values in these tables can be doubled, or the design loads on the I-Joist may be halved to verify the capacity of each ply. The capacity is additive. See Double I-Joist Connection detail on page 25.
4. Web stiffeners are not required for these spans and loads.
5. Web fillers are required for I-Joists seated in hangers that do not laterally support the top flange or for hangers that require nailing into the web
6. Where the Live Load is blank, the Total Load governs the design.
7. Do not use a product where designated "-" without further analysis by a design professional

PSF TO PLF CONVERSION

| OC Spacing | Load |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 20 psf | 25 psf | 30 psf | 35 psf | 40 psf | 45 psf | 50 psf | 55 psf | 60 psf | 65 psf |
| 12" | 20 | 25 | 30 | 35 | 40 | 45 | 50 | 55 | 60 | 65 |
| 16 " | 27 | 34 | 40 | 47 | 54 | 60 | 67 | 74 | 80 | 87 |
| 19.2" | 32 | 40 | 48 | 56 | 64 | 72 | 80 | 88 | 96 | 104 |
| 24" | 40 | 50 | 60 | 70 | 80 | 90 | 100 | 110 | 120 | 130 |

## TO USE:

1. Select the span required
2. Compare the design total load to the Total Load column.
3. Compare the design live load to the appropriate Live Load column.
4. Select a product that exceeds both the design total and live loads.

## EXAMPLE:

Select an I-Joist for a 27'-6" clear span supporting 40 psf Live Load and 20 psf Dead Load, spaced 16 " oc, at an L/480 deflection limit

1. Select the row corresponding to an 28 ' span.
2. Design Total Load $=(40+20) *(16 / 12)=80$ plf

Design Live Load $=40$ * (16 / 12) $=54$ plf
3. Select the first joist to exceed both Total Load and L/480:

The 16" LPI 52Plus supports 97 plf Total Load and 54 plf Live Load at L/480

| Span | 14" LPI 18 |  |  | 14" LPI 20 Plus |  |  | 14" LPI 32Plus |  |  | 14" LPI 36 |  |  | 14" LPI 42Plus |  |  | 14" LPI 52Plus |  |  | 14" LPI 56 |  |  | Span |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Live Load |  | $\begin{aligned} & \text { Total } \\ & \text { Load } \end{aligned}$ | Live Load |  | Total Load | Live Load |  | TotalLoad | Live Load |  | Total Load | Live Load |  | Total Load | Live Load |  | Total Load | Live Load |  | Total Load |  |
|  | L/480 | L/360 |  | L/480 | L/360 |  | L/480 | L/360 |  | L/480 | L/360 |  | L/480 | L/360 |  | L/480 | L/360 |  | L/480 | L/360 |  |  |
| 14' |  |  | 123 |  |  | 137 |  |  | 137 |  |  | 139 |  |  | 175 |  |  | 192 |  |  | 165 | 14' |
| 15' | 103 |  | 115 |  |  | 128 |  |  | 128 |  |  | 130 |  |  | 164 |  |  | 179 |  |  | 154 | 15' |
| 16' | 87 |  | 108 | 108 |  | 120 | 117 |  | 120 |  |  | 122 |  |  | 154 |  |  | 168 |  |  | 145 | 16' |
| 17' | 73 | 98 | 100 | 91 |  | 113 | 101 |  | 113 |  |  | 115 |  |  | 145 |  |  | 158 |  |  | 136 | 17' |
| 18' | 62 | 83 | 89 | 78 | 104 | 106 | 87 |  | 107 | 100 |  | 109 | 126 |  | 137 | 140 |  | 150 |  |  | 129 | 18' |
| 19' | 53 | 71 | 80 | 67 | 89 | 95 | 75 | 101 | 101 | 86 |  | 103 | 109 |  | 130 | 121 |  | 142 |  |  | 122 | 19' |
| 20' | 46 | 62 | 72 | 58 | 77 | 86 | 66 | 88 | 96 | 75 |  | 98 | 95 |  | 123 | 105 |  | 135 | 112 |  | 116 | 20' |
| 21' | 40 | 54 | 66 | 51 | 68 | 78 | 58 | 77 | 92 | 65 | 87 | 93 | 83 | 111 | 117 | 92 | 122 | 129 | 98 |  | 111 | 21 |
| 22' | 35 | 47 | 60 | 44 | 59 | 71 | 51 | 68 | 88 | 57 | 76 | 89 | 73 | 97 | 112 | 80 | 107 | 123 | 87 |  | 106 | $22^{\prime}$ |
| 23' | 31 | 41 | 55 | 39 | 52 | 65 | 45 | 60 | 83 | 50 | 67 | 85 | 64 | 86 | 107 | 71 | 95 | 117 | 77 |  | 101 | 23' |
| 24' | 27 | 36 | 50 | 34 | 46 | 60 | 40 | 54 | 76 | 45 | 60 | 82 | 57 | 76 | 103 | 63 | 84 | 113 | 68 | 91 | 97 | $24^{\prime}$ |
| 25' | 24 | 32 | 46 | 31 | 41 | 55 | 36 | 48 | 71 | 40 | 53 | 79 | 51 | 68 | 99 | 56 | 75 | 108 | 61 | 81 | 93 | $25^{\prime}$ |
| 26' | 22 | 29 | 43 | 27 | 37 | 51 | 32 | 43 | 65 | 36 | 48 | 72 | 45 | 61 | 91 | 50 | 67 | 100 | 54 | 73 | 90 | 26' |
| 27' | 19 | 26 | 39 | 24 | 33 | 47 | 29 | 39 | 58 | 32 | 43 | 64 | 41 | 55 | 82 | 45 | 60 | 90 | 49 | 65 | 86 | 27' |
| 28' | 17 | 23 | 35 | 22 | 29 | 44 | 26 | 35 | 53 | 29 | 38 | 58 | 37 | 49 | 74 | 40 | 54 | 81 | 44 | 59 | 83 | $28^{\prime}$ |
| 29' | 16 | 21 | 32 | 20 | 27 | 40 | 24 | 32 | 48 | 26 | 35 | 52 | 33 | 44 | 67 | 36 | 49 | 73 | 40 | 53 | 80 | $29^{\prime}$ |
| 30' | 14 | 19 | 29 | 18 | 24 | 36 | 21 | 29 | 43 | 23 | 31 | 47 | 30 | 40 | 61 | 33 | 44 | 67 | 36 | 48 | 73 | $30^{\prime}$ |
| 31' | 13 | 17 | 26 | 16 | 22 | 33 | 19 | 26 | 39 | - | - | - | 27 | 37 | 55 | 30 | 40 | 61 | 33 | 44 | 66 | 31' |
| 32' | - | - | - | - | - | - | - | - | - | - | - | - | 25 | 33 | 50 | 27 | 37 | 55 | 30 | 40 | 60 | 32' |
| 33' | - | - | - | - | - | - | - | - | - | - | - | - | 23 | 30 | 46 | 25 | 33 | 50 | 27 | 37 | 55 | 33' |
| 34' | - | - | - | - | - | - | - | - | - | - | - | - | 21 | 28 | 42 | 23 | 31 | 46 | 25 | 34 | 51 | 34' |


| Span | 16" LPI 20Plus |  |  | 16" LPI 32Plus |  |  | 16" LPI 36 |  |  | 16" LPI 42Plus |  |  | 16" LPI 52Plus |  |  | 16" LPI 56 |  |  | Span |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Live Load |  | Total Load | Live Load |  | Total Load | Live Load |  | Total Load | Live Load |  | Total Load | Live Load |  | Total Load | Live Load |  | Total Load |  |
|  | L/480 | L/360 |  | L/480 | L/360 |  | L/480 | L/360 |  | L/480 | L/360 |  | L/480 | L/360 |  | L/480 | L/360 |  |  |
| 14' |  |  | 139 |  |  | 139 |  |  | 139 |  |  | 181 |  |  | 193 |  |  | 165 | 14' |
| 15' |  |  | 130 |  |  | 130 |  |  | 130 |  |  | 169 |  |  | 180 |  |  | 155 | 15' |
| 16' |  |  | 122 |  |  | 122 |  |  | 122 |  |  | 159 |  |  | 169 |  |  | 145 | 16' |
| 17' |  |  | 115 |  |  | 115 |  |  | 115 |  |  | 150 |  |  | 159 |  |  | 137 | 17' |
| 18' | 105 |  | 109 |  |  | 109 |  |  | 109 |  |  | 141 |  |  | 150 |  |  | 129 | 18' |
| 19' | 90 |  | 103 | 98 |  | 103 |  |  | 103 |  |  | 134 |  |  | 143 |  |  | 122 | 19' |
| $20^{\prime}$ | 78 |  | 98 | 86 |  | 98 |  |  | 98 | 126 |  | 127 |  |  | 136 |  |  | 116 | 20' |
| 21 | 68 |  | 89 | 76 |  | 93 | 86 |  | 93 | 110 |  | 121 | 122 |  | 129 |  |  | 111 | 21' |
| 22' | 60 | 80 | 81 | 67 |  | 89 | 76 |  | 89 | 97 |  | 116 | 107 |  | 123 |  |  | 106 | 22' |
| 23' | 53 | 71 | 74 | 59 | 79 | 85 | 67 |  | 85 | 86 |  | 111 | 95 |  | 118 | 101 |  | 101 | 23' |
| 24' | 47 | 63 | 68 | 53 | 71 | 82 | 59 | 79 | 82 | 76 | 102 | 106 | 84 | 112 | 113 | 90 |  | 97 | 24' |
| $25^{\prime}$ | 42 | 56 | 63 | 47 | 63 | 78 | 53 | 71 | 79 | 68 | 91 | 102 | 75 | 100 | 109 | 80 |  | 93 | 25' |
| 26' | 37 | 50 | 58 | 42 | 57 | 75 | 47 | 63 | 76 | 61 | 82 | 98 | 67 | 89 | 104 | 72 |  | 90 | 26' |
| 27 | 33 | 45 | 54 | 38 | 51 | 70 | 42 | 57 | 73 | 55 | 73 | 95 | 60 | 80 | 101 | 65 |  | 86 | 27' |
| 28' | 30 | 40 | 50 | 35 | 46 | 65 | 38 | 51 | 70 | 49 | 66 | 91 | 54 | 72 | 97 | 58 | 78 | 83 | 28' |
| 29' | 27 | 36 | 47 | 31 | 42 | 61 | 35 | 46 | 68 | 45 | 60 | 88 | 49 | 65 | 94 | 53 | 71 | 80 | 29' |
| 30' | 25 | 33 | 44 | 28 | 38 | 57 | 31 | 42 | 63 | 41 | 54 | 82 | 44 | 59 | 89 | 48 | 64 | 78 | 30' |
| 31' | 22 | 30 | 41 | 26 | 35 | 52 | 28 | 38 | 57 | 37 | 49 | 74 | 40 | 54 | 81 | 44 | 58 | 75 | 31' |
| 32' | 20 | 27 | 38 | 24 | 32 | 48 | 26 | 35 | 52 | 34 | 45 | 68 | 37 | 49 | 74 | 40 | 53 | 73 | 32' |
| 33' | 19 | 25 | 36 | 22 | 29 | 44 | 24 | 32 | 48 | 31 | 41 | 62 | 34 | 45 | 68 | 37 | 49 | 71 | 33' |
| 34' | 17 | 23 | 34 | 20 | 27 | 40 | 22 | 29 | 44 | 28 | 38 | 57 | 31 | 41 | 62 | 33 | 45 | 67 | 34' |

## DESIGN ASSUMPTIONS:

1. Span is the clear distance between supports and is valid for simple or continuous span applications. Continuous spans are based on the longest span. The shortest span shal not be less than $50 \%$ of the longest span.
2. The values in the tables are for uniform loads only.
3. Total Load is for normal ( $100 \%$ ) duration
4. These tables do not reflect any additional stiffness provided by the floor sheathing.
5. Live Load deflection is limited to $L / 360$ or $L / 480$ as noted in the table.
6. Total Load deflection is limited to $\mathrm{L} / 240$. Long term deflection (creep) has not been considered.
7. These tables assume full lateral support of the compression flange (maximum unbraced length of 24.')
8. These tables are based on an end bearing length of at least $1-3 / 4$ " and an interior bearing length of at least $3-1 / 2$," and are limited to the bearing capacity for an SPF wall plate ( $\left.\mathrm{F}_{\mathrm{c} \perp}=425 \mathrm{psi}\right)$.

## ADDITIONAL NOTES:

1. The allowable loads represent the capacity of the joist in pounds per lineal foot (plf) of length.
2. The designer shall check both the Total Load and the appropriate Live Load column.
3. To design a double l-Joist, the values in these tables can be doubled, or the design loads on the I-Joist may be halved to verify the capacity of each ply. The capacity is additive. See Double -Joist Connection detail on page 25
4. Web stiffeners are not required for these spans and loads.
5. Web fillers are required for I-Joists seated in hangers that do not laterally support the top flange or for hangers that require nailing into the web.
6. Where the Live Load is blank, the Total Load governs the design.
7. Do not use a product where designated "-" without further analysis by a design professional

PSF TO PLF CONVERSION

| OC Spacing | Load |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 20 psf | 25 psf | 30 psf | 35 psf | 40 psf | 45 psf | 50 psf | 55 psf | 60 psf | 65 psf |
| 12" | 20 | 25 | 30 | 35 | 40 | 45 | 50 | 55 | 60 | 65 |
| 16 " | 27 | 34 | 40 | 47 | 54 | 60 | 67 | 74 | 80 | 87 |
| 19.2" | 32 | 40 | 48 | 56 | 64 | 72 | 80 | 88 | 96 | 104 |
| 24" | 40 | 50 | 60 | 70 | 80 | 90 | 100 | 110 | 120 | 130 |

## TO USE:

1. Select the span required. For roofs with a pitch of $2: 12$ or greater, the horizontal span shall be multiplied by the appropriate roof pitch adjustment factor from the table at the bottom of this page
2. Compare the design total load to the appropriate Total Load column for Snow (115\%) or Non-Snow (125\%)
. Compare the design live load to the Live Load column (L/240). For live load deflection limits of L/480 or L/360, compare the design live load to the appropriate Live Load column from the Uniform Floor Load Tables on pages 8 and 9
3. Select a product that exceeds both the design total and live loads.

## EXAMPLE:

Select an I-Joist for a 12'-8" horizontal clear span supporting 45 psf Snow (Live) Load and 15 psf Dead Load, spaced 24" oc, with a roof slope of 6:12, at an L/240 deflection limit.

1. Sloped Span $=(12+8 / 12)^{*} 1.118=14.16^{\prime} \rightarrow$ use $15^{\prime}$
2. Design Total Load $=(45+15)^{*}(24 / 12)=120$ plf Design Live Load $=45^{*}(24 / 12)=90$ plf
3. Select the row corresponding to a 15 ' span.
4. Select the first I-Joist to exceed both Total Load for Snow (115\%) and L/240

The 9-1/2" LPI 32Plus supports 132 plf Total Load (Snow 115\%) and 127 plf Live Load at L/240

| Span | 9-1/2" LPI 18 |  |  | 9-1/2" LPI 20Plus |  |  | 9-1/2" LPI 32Plus |  |  | 9-1/2" LPI 42Plus |  |  | Span |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Live <br> Load <br> L/240 | Total Load |  | Live Load L/240 | Total Load |  | Live Load L/240 | Total Load |  | Live Load <br> L/240 | Total Load |  |  |
|  |  | $\begin{aligned} & \text { Snow } \\ & \text { 115\% } \end{aligned}$ | NonSnow 125\% |  | Snow 115\% | NonSnow 125\% |  | Snow 115\% | Non- Snow $125 \%$ |  | Snow 115\% | $\begin{aligned} & \text { Non- } \\ & \text { Snow } \\ & 125 \% \end{aligned}$ |  |
| 8' |  | 219 | 238 |  | 243 | 264 |  | 243 | 264 |  | 299 | 326 | 8' |
| 9' |  | 195 | 212 |  | 217 | 236 |  | 217 | 236 |  | 267 | 290 | 9' |
| 10' |  | 176 | 191 |  | 196 | 213 |  | 196 | 213 |  | 241 | 262 | 10' |
| 11' |  | 160 | 174 |  | 178 | 194 |  | 178 | 194 |  | 219 | 238 | 11' |
| 12' | 157 | 145 | 158 |  | 164 | 178 |  | 164 | 178 |  | 201 | 219 | 12' |
| $13^{\prime}$ | 125 | 124 | 135 | 159 | 147 | 160 |  | 151 | 165 |  | 186 | 202 | $13^{\prime}$ |
| 14' | 102 | 107 | 116 | 130 | 127 | 138 | 152 | 141 | 153 |  | 173 | 188 | 14' |
| 15' | 84 | 93 | 102 | 107 | 111 | 121 | 127 | 132 | 143 |  | 162 | 176 | 15' |
| $16^{\prime}$ | 70 | 82 | 89 | 89 | 98 | 106 | 107 | 123 | 134 | 149 | 152 | 165 | $16^{\prime}$ |
| 17' | 59 | 73 | 78 | 75 | 87 | 94 | 91 | 112 | 121 | 126 | 143 | 155 | 17' |
| 18' | 50 | 65 | 66 | 64 | 77 | 84 | 78 | 100 | 104 | 108 | 135 | 144 | 18' |
| 19' | 42 | 57 | 57 | 55 | 69 | 73 | 67 | 90 | 90 | 92 | 123 | 123 | 19' |
| $20^{\prime}$ | 36 | 49 | 49 | 47 | 63 | 63 | 58 | 78 | 78 | 80 | 107 | 107 | 20' |
| 21' | 32 | 42 | 42 | 41 | 55 | 55 | 51 | 68 | 68 | 70 | 93 | 93 | 21' |
| 22' | 28 | 37 | 37 | 36 | 48 | 48 | 45 | 60 | 60 | 61 | 81 | 81 | 22' |
| 23' | 24 | 32 | 32 | 31 | 42 | 42 | 39 | 53 | 53 | 54 | 72 | 72 | 23' |
| 24' | 21 | 29 | 29 | 28 | 37 | 37 | 35 | 47 | 47 | 47 | 63 | 63 | 24' |
| 25' | 19 | 25 | 25 | 24 | 33 | 33 | 31 | 41 | 41 | 42 | 56 | 56 | 25' |
| 26' | 17 | 22 | 22 | 22 | 29 | 29 | 28 | 37 | 37 | 37 | 50 | 50 | $26^{\prime}$ |
| 27' | 15 | 20 | 20 | 19 | 26 | 26 | 25 | 33 | 33 | 34 | 45 | 45 | 27' |
| 28' | 13 | 18 | 18 | 17 | 23 | 23 | 22 | 30 | 30 | 30 | 40 | 40 | 28' |


| Span | 11-7/8" LPI 18 |  |  | 11-7/8" LPI 20Plus |  |  | 11-7/8" LPI 32Plus |  |  | 11-7/8" LPI 36 |  |  | 11-7/8" LPI 42Plus |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Live <br> Load <br> L/240 | Total Load |  | Live <br> Load <br> L/240 | Total Load |  | Live Load <br> L/240 | Total Load |  | Live Load L/240 | Total Load |  | Live Load L/240 | Total Load |  |
|  |  | $\begin{aligned} & \text { Snow } \\ & \text { 115\% } \end{aligned}$ | Non- Snow $125 \%$ |  | $\begin{aligned} & \text { Snow } \\ & \text { 115\% } \end{aligned}$ | NonSnow 125\% |  | $\begin{aligned} & \text { Snow } \\ & \text { 115\% } \end{aligned}$ | NonSnow 125\% |  | $\begin{aligned} & \text { Snow } \\ & \text { 115\% } \end{aligned}$ | NonSnow 125\% |  | $\begin{aligned} & \text { Snow } \\ & \text { 115\% } \end{aligned}$ | $\begin{aligned} & \text { Non- } \\ & \text { Snow } \\ & 125 \% \end{aligned}$ |
| 8' |  | 232 | 252 |  | 258 | 281 |  | 258 | 281 |  | 277 | 301 |  | 335 | 364 |
| 9' |  | 207 | 225 |  | 230 | 250 |  | 230 | 250 |  | 247 | 269 |  | 299 | 325 |
| 10' |  | 187 | 203 |  | 208 | 226 |  | 208 | 226 |  | 223 | 242 |  | 270 | 293 |
| 11' |  | 170 | 185 |  | 189 | 206 |  | 189 | 206 |  | 203 | 221 |  | 246 | 267 |
| $12^{\prime}$ |  | 156 | 170 |  | 174 | 189 |  | 174 | 189 |  | 187 | 203 |  | 226 | 246 |
| $13^{\prime}$ |  | 145 | 157 |  | 161 | 175 |  | 161 | 175 |  | 173 | 188 |  | 209 | 227 |
| 14' |  | 134 | 146 |  | 149 | 163 |  | 149 | 163 |  | 160 | 174 |  | 194 | 211 |
| 15' |  | 123 | 133 |  | 140 | 152 |  | 140 | 152 |  | 150 | 163 |  | 181 | 197 |
| 16' |  | 108 | 117 |  | 131 | 142 |  | 131 | 143 |  | 141 | 153 |  | 170 | 185 |
| $17^{\prime}$ | 100 | 96 | 104 | 126 | 116 | 126 |  | 123 | 134 |  | 133 | 144 |  | 160 | 174 |
| 18' | 85 | 85 | 93 | 107 | 104 | 113 | 125 | 117 | 127 |  | 125 | 136 |  | 152 | 165 |
| 19' | 73 | 77 | 83 | 92 | 93 | 101 | 108 | 111 | 120 | 122 | 119 | 129 | 153 | 144 | 156 |
| $20^{\prime}$ | 63 | 69 | 75 | 80 | 84 | 91 | 94 | 105 | 114 | 105 | 113 | 123 | 133 | 137 | 149 |
| 21' | 55 | 63 | 68 | 69 | 76 | 83 | 82 | 95 | 104 | 92 | 108 | 117 | 116 | 130 | 142 |
| 22' | 48 | 57 | 62 | 61 | 69 | 76 | 73 | 87 | 95 | 81 | 103 | 108 | 102 | 124 | 135 |
| 23' | 42 | 52 | 56 | 53 | 64 | 69 | 64 | 80 | 86 | 71 | 95 | 95 | 90 | 118 | 120 |
| 24' | 37 | 48 | 50 | 47 | 58 | 63 | 57 | 73 | 76 | 63 | 84 | 84 | 79 | 106 | 106 |
| 25' | 33 | 44 | 44 | 42 | 54 | 56 | 51 | 67 | 68 | 56 | 75 | 75 | 71 | 94 | 94 |
| 26' | 29 | 39 | 39 | 37 | 50 | 50 | 45 | 61 | 61 | 50 | 67 | 67 | 63 | 84 | 84 |
| 27' | 26 | 35 | 35 | 33 | 45 | 45 | 41 | 55 | 55 | 45 | 60 | 60 | 57 | 76 | 76 |
| 28' | 23 | 31 | 31 | 30 | 40 | 40 | 37 | 49 | 49 | 40 | 54 | 54 | 51 | 68 | 68 |



## DESIGN ASSUMPTIONS:

1. Span is the clear distance between supports, along the sloped length of the joist, for simple or continuous span applications. Continuous spans are based on the longest span. The shortest span shall not be less than $50 \%$ of the longest span
2. The values in the tables are for uniform loads only and represent the design gravity oads, unadjusted for roof pitch. Wind uplift has not been considered
3. Total Load is for Snow (115\%) or Non-Snow (125\%) duration as noted
4. These tables do not reflect any additional stiffness provided by the roof sheathing.
5. Total Load deflection is limited to $\mathrm{L} / 180$. Long term deflection (creep) has not been considered.
6. These tables assume full lateral support of the compression flange (maximum unbraced length of 24 ")
7. These tables are based on an end bearing length of at least $1-3 / 4$ " and an interior bearing length of at least $3-1 / 2$," and are limited to the bearing capacity for an SPF wall plate ( $\mathrm{F}_{\mathrm{c} \mathrm{\perp}}=425 \mathrm{psi}$ ).

## ADDITIONAL NOTES:

. The allowable loads represent the capacity of the joist in pounds per lineal foot (plf) of length
2. The designer shall check the appropriate Total Load and the Live Load column
3. For roofs with a pitch of $2: 12$ or greater, the horizontal span shall be multiplied by the appropriate adjustment factor from the table at the bottom of this page.
4. To design a double I-Joist, the values in these tables can be doubled, or the design loads on the I-Joist may be halved to verify the capacity of each ply. The capacity is additive. See Double 1 -Joist Connection detail on page 25.
5. Web stiffeners are not required for these spans and loads.
6. Web fillers are required for I-Joists seated in hangers that do not laterally support the top flange or for hangers that require nailing into the web
7. Where the Live Load is blank, the Total Load governs the design.
8. Where the Total Load for Snow and Non-Snow is the same, the design is controlled either by bearing on the SPF wall plate or by L/180 total load deflection. No increase for load duration is allowed in either case.
9. To design for a live load deflection limit of L/480 or L/360, use the Uniform Floor Load tables.

## TO USE:

1. Select the span required. For roofs with a pitch of $2: 12$ or greater, the horizontal span shall be multiplied by the appropriate roof pitch adjustment factor from the table at the bottom of this page
2. Compare the design total load to the appropriate Total Load column for Snow (115\%) or Non-Snow (125\%)
3. Compare the design live load to the Live Load column (L/240). For live load deflection limits of L/480 or L/360, compare the design live load to the appropriate Live Load column from the Uniform Floor Load Tables on pages 8 and 9
4. Select a product that exceeds both the design total and live loads.

EXAMPLE:
Select an I-Joist for a 23'-3" horizontal clear span supporting 45 psf Snow (Live) Load and 15 psf Dead Load, spaced 24" oc, with a roof slope of $6: 12$, at an $\mathrm{L} / 240$ deflection limit.

1. Sloped Span $=(23+3 / 12) * 1.118=25.99 ' \longrightarrow$ use 26
2. Design Total Load $=(45+15) *(24 / 12)=120$ plf Design Live Load $=45$ * (24 / 12) $=90$ plf
3. Select the row corresponding to a 26 ' span.
4. Select the first I-Joist to exceed both Total Load for Snow (115\%) and L/240: The 14" LPI 52Plus supports 120 plf Total Load (Snow $115 \%$ ) and 100 plf Live Load at L/240

| Span | 14" LPI 18 |  |  | 14" LPI 20Plus |  |  | 14" LPI 32Plus |  |  | 14" LPI 36 |  |  | 14" LPI 42Plus |  |  | 14" LPI 52Plus |  |  | 14" LPI 56 |  |  | Span |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Live Load L/240 | Total Load |  | Live Load L/240 | Total Load |  | Live <br> Load <br> L/240 | Total Load |  | Live Load L/240 | Total Load |  | LiveLoadL/240 | Total Load |  | Live Load <br> L/240 | Total Load |  | Live Load L/240 | Total Load |  |  |
|  |  | $\begin{aligned} & \text { Snow } \\ & \text { 115\% } \end{aligned}$ | NonSnow 125\% |  | $\begin{aligned} & \text { Snow } \\ & \text { 115\% } \end{aligned}$ | NonSnow 125\% |  | $\begin{aligned} & \text { Snow } \\ & 115 \% \end{aligned}$ | Non- Snow $125 \%$ |  | $\begin{aligned} & \text { Snow } \\ & \text { 115\% } \end{aligned}$ | NonSnow 125\% |  | Snow 115\% | NonSnow 125\% |  | $\begin{aligned} & \text { Snow } \\ & \text { 115\% } \end{aligned}$ | NonSnow 125\% |  | $\begin{aligned} & \text { Snow } \\ & 115 \% \end{aligned}$ | NonSnow <br> 125\% |  |
| 14' |  | 141 | 154 |  | 158 | 171 |  | 158 | 171 |  | 160 | 174 |  | 202 | 219 |  | 221 | 240 |  | 190 | 207 | $14{ }^{\prime}$ |
| 15' |  | 132 | 144 |  | 147 | 160 |  | 147 | 160 |  | 150 | 163 |  | 188 | 205 |  | 206 | 224 |  | 178 | 193 | 15' |
| 16' |  | 124 | 135 |  | 138 | 150 |  | 138 | 150 |  | 141 | 153 |  | 177 | 192 |  | 193 | 210 |  | 167 | 181 | 16' |
| 17' |  | 115 | 125 |  | 130 | 141 |  | 130 | 141 |  | 133 | 144 |  | 167 | 181 |  | 182 | 198 |  | 157 | 171 | $17{ }^{\prime}$ |
| 18' |  | 103 | 112 |  | 121 | 132 |  | 123 | 134 |  | 125 | 136 |  | 157 | 171 |  | 172 | 187 |  | 148 | 161 | 18' |
| 19' |  | 92 | 100 |  | 109 | 119 |  | 117 | 127 |  | 119 | 129 |  | 149 | 162 |  | 163 | 178 |  | 141 | 153 | 19' |
| 20' |  | 83 | 90 |  | 99 | 107 |  | 111 | 120 |  | 113 | 123 |  | 142 | 154 |  | 155 | 169 |  | 134 | 145 | 20' |
| 21' | 81 | 76 | 82 |  | 89 | 97 |  | 106 | 115 |  | 108 | 117 |  | 135 | 147 |  | 148 | 161 |  | 127 | 139 | 21' |
| 22' | 71 | 69 | 75 |  | 81 | 89 | 102 | 101 | 110 |  | 103 | 112 |  | 129 | 140 |  | 141 | 154 |  | 122 | 132 | 22' |
| 23' | 62 | 63 | 69 | 78 | 75 | 81 | 91 | 96 | 104 | 101 | 98 | 107 | 129 | 124 | 134 | 142 | 135 | 147 |  | 116 | 127 | 23' |
| 24' | 55 | 58 | 63 | 69 | 69 | 75 | 81 | 88 | 96 | 90 | 94 | 102 | 115 | 118 | 129 | 126 | 130 | 141 |  | 112 | 121 | 24' |
| 25' | 49 | 53 | 58 | 62 | 63 | 69 | 72 | 81 | 88 | 80 | 90 | 98 | 102 | 114 | 124 | 112 | 124 | 135 |  | 107 | 117 | 25' |
| 26' | 44 | 49 | 54 | 55 | 58 | 64 | 65 | 75 | 82 | 72 | 87 | 95 | 91 | 109 | 119 | 100 | 120 | 130 | 109 | 103 | 112 | 26' |
| 27' | 39 | 46 | 50 | 49 | 54 | 59 | 58 | 70 | 76 | 64 | 84 | 86 | 82 | 104 | 110 | 90 | 115 | 120 | 98 | 99 | 108 | 27' |
| 28' | 35 | 42 | 46 | 44 | 50 | 55 | 53 | 65 | 70 | 58 | 77 | 77 | 74 | 96 | 99 | 81 | 108 | 108 | 88 | 96 | 104 | 28' |
| 29' | 32 | 40 | 42 | 40 | 47 | 51 | 48 | 60 | 64 | 52 | 70 | 70 | 67 | 89 | 89 | 73 | 98 | 98 | 80 | 92 | 101 | 29' |
| 30' | 29 | 37 | 38 | 36 | 44 | 48 | 43 | 56 | 58 | 47 | 63 | 63 | 61 | 81 | 81 | 67 | 89 | 89 | 73 | 89 | 97 | 30' |
| 31' | 26 | 35 | 35 | 33 | 41 | 44 | 39 | 53 | 53 | 43 | 57 | 57 | 55 | 74 | 74 | 61 | 81 | 81 | 66 | 87 | 88 | 31' |
| 32' | 24 | 32 | 32 | 30 | 38 | 40 | 36 | 48 | 48 | 39 | 52 | 52 | 50 | 67 | 67 | 55 | 74 | 74 | 60 | 81 | 81 | 32' |
| 33' | 21 | 29 | 29 | 27 | 36 | 37 | 33 | 44 | 44 | 36 | 48 | 48 | 46 | 61 | 61 | 50 | 67 | 67 | 55 | 74 | 74 | 33' |
| 34' | 20 | 26 | 26 | 25 | 34 | 34 | 30 | 40 | 40 | 33 | 44 | 44 | 42 | 56 | 56 | 46 | 62 | 62 | 51 | 68 | 68 | 34' |


| Span | 16" LPI 20Plus |  |  | 16" LPI 32Plus |  |  | 16" LPI 36 |  |  | 16" LPI 42Plus |  |  | 16" LPI 52Plus |  |  | 16" LPI 56 |  |  | Span |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Live <br> L/240 | Total Load |  | Live Load L/240 | Total Load |  | $\begin{aligned} & \text { Live } \\ & \text { Load } \\ & \text { L/240 } \end{aligned}$ | Total Load |  | Live Load L/240 | Total Load |  | $\begin{aligned} & \text { Live } \\ & \text { Load } \\ & \text { L/240 } \end{aligned}$ | Total Load |  | Live Load L/240 | Total Load |  |  |
|  |  | $\begin{aligned} & \text { Snow } \\ & \text { 115\% } \end{aligned}$ | Non- Snow $125 \%$ |  | $\begin{aligned} & \text { Snow } \\ & \text { 115\% } \end{aligned}$ | Non- Snow $125 \%$ |  | Snow | Non- Snow $125 \%$ |  | Snow 115\% | Non- Snow $125 \%$ |  | Snow 115\% | Non- Snow $125 \%$ |  | Snow 115\% | Non- Snow $125 \%$ |  |
| 14' |  | 160 | 174 |  | 160 | 174 |  | 160 | 174 |  | 208 | 227 |  | 222 | 241 |  | 190 | 207 | 14' |
| 15' |  | 149 | 162 |  | 149 | 162 |  | 150 | 163 |  | 195 | 212 |  | 207 | 225 |  | 178 | 193 | 15' |
| $16^{\prime}$ |  | 140 | 152 |  | 140 | 152 |  | 141 | 153 |  | 183 | 199 |  | 194 | 211 |  | 167 | 181 | 16' |
| 17' |  | 132 | 144 |  | 132 | 144 |  | 133 | 144 |  | 172 | 187 |  | 183 | 199 |  | 157 | 171 | 17' |
| 18' |  | 125 | 136 |  | 125 | 136 |  | 125 | 136 |  | 163 | 177 |  | 173 | 188 |  | 149 | 162 | 18' |
| 19' |  | 118 | 129 |  | 118 | 129 |  | 119 | 129 |  | 154 | 168 |  | 164 | 178 |  | 141 | 153 | 19' |
| 20' |  | 113 | 122 |  | 113 | 122 |  | 113 | 123 |  | 147 | 159 |  | 156 | 170 |  | 134 | 146 | 20' |
| 21' |  | 103 | 112 |  | 107 | 117 |  | 108 | 117 |  | 140 | 152 |  | 149 | 162 |  | 128 | 139 | 21' |
| 22' |  | 94 | 102 |  | 102 | 111 |  | 103 | 112 |  | 133 | 145 |  | 142 | 154 |  | 122 | 133 | 22' |
| 23' |  | 86 | 93 |  | 98 | 107 |  | 98 | 107 |  | 128 | 139 |  | 136 | 148 |  | 117 | 127 | 23' |
| 24' |  | 79 | 86 |  | 94 | 102 |  | 94 | 102 |  | 122 | 133 |  | 130 | 142 |  | 112 | 122 | 24' |
| 25' |  | 73 | 79 | 95 | 90 | 98 |  | 90 | 98 |  | 118 | 128 |  | 125 | 136 |  | 107 | 117 | 25' |
| $26^{\prime}$ |  | 67 | 73 | 85 | 87 | 94 |  | 87 | 95 | 123 | 113 | 123 |  | 120 | 131 |  | 103 | 112 | 26' |
| 27' | 67 | 62 | 68 | 77 | 81 | 88 | 85 | 84 | 91 | 110 | 109 | 118 | 121 | 116 | 126 |  | 99 | 108 | $27^{\prime}$ |
| 28' | 61 | 58 | 63 | 70 | 75 | 82 | 77 | 81 | 88 | 99 | 105 | 114 | 109 | 112 | 121 |  | 96 | 104 | 28' |
| 29' | 55 | 54 | 59 | 63 | 70 | 76 | 70 | 78 | 85 | 90 | 101 | 110 | 98 | 108 | 117 |  | 93 | 101 | 29' |
| $30^{\prime}$ | 50 | 50 | 55 | 57 | 65 | 71 | 63 | 75 | 82 | 82 | 97 | 106 | 89 | 104 | 113 | 97 | 90 | 97 | 30' |
| 31' | 45 | 47 | 51 | 52 | 61 | 67 | 57 | 73 | 77 | 74 | 91 | 99 | 81 | 101 | 109 | 88 | 87 | 94 | 31' |
| 32' | 41 | 44 | 48 | 48 | 58 | 63 | 52 | 70 | 70 | 68 | 86 | 91 | 74 | 98 | 99 | 80 | 84 | 91 | 32' |
| 33' | 38 | 42 | 45 | 44 | 54 | 59 | 48 | 64 | 64 | 62 | 81 | 83 | 68 | 91 | 91 | 74 | 81 | 89 | 33' |
| 34' | 34 | 39 | 43 | 40 | 51 | 54 | 44 | 59 | 59 | 57 | 76 | 76 | 62 | 83 | 83 | 67 | 79 | 86 | 34' |

## DESIGN ASSUMPTIONS:

1. Span is the clear distance between supports, along the sloped length of the joist, for simple or continuous span applications. Continuous spans are based on the longest span. The shortest span shall not be less than $50 \%$ of the longest span.
2. The values in the tables are for uniform loads only and represent the design gravity loads, unadjusted for roof pitch. Wind uplift has not been considered.
3. Total Load is for Snow ( $115 \%$ ) or Non-Snow ( $125 \%$ ) duration as noted.
4. These tables do not reflect any additional stiffness provided by the roof sheathing.
5. Total Load deflection is limited to L/180. Long term deflection (creep) has not been considered.
6. These tables assume full lateral support of the compression flange (maximum unbraced length of $24^{\prime \prime}$ ).
7. These tables are based on an end bearing length of at least $1-3 / 4^{\prime \prime}$ and an interior bearing length of at least $3-1 / 2$, and are limited to the bearing capacity for an SPF wall plate ( $\mathrm{F}_{\mathrm{c} \perp}=425 \mathrm{psi}$ ).

## ADDITIONAL NOTES

1. The allowable loads represent the capacity of the joist in pounds per lineal foot (plf) of length.
2. The designer shall check the appropriate Total Load and the Live Load column.
3. For roofs with a pitch of $2: 12$ or greater, the horizontal span shall be multiplied by the appropriate adjustment factor from the table at the bottom of this page.
4. To design a double I-Joist, the values in these tables can be doubled, or the design loads on the I-Joist may be halved to verify the capacity of each ply. The capacity is additive. See Double 1 -oist Connection detail on page 25 .
5. Web stiffeners are not required for these spans and loads.
6. Web fillers are required for I -Joists seated in hangers that do not laterally support the top flange or for hangers that require nailing into the web.
7. Where the Live Load is blank, the Total Load governs the design.
8. Where the Total Load for Snow and Non-Snow is the same, the design is controlled either by bearing on the SPF wall plate or by L/180 total load deflection. No increase for load duration is allowed in either case.
9. To design for a live load deflection limit of $\mathrm{L} / 480$ or $\mathrm{L} / 360$, use the Uniform Floor Load tables.

## TO USE:

1. Select the appropriate set of tables based on roof pitch.
2. Select the section of that table that corresponds to the design roof live load (snow or non-snow).
3. Find a span that meets or exceeds the design span for the appropriate roof dead load ( 15 psf or 20 psf ).
4. Read the corresponding series, depth and spacing

## DESIGN ASSUMPTIONS:

1. The spans listed are the horizontal clear distance between supports and are valid for simple or continuous span applications. Continuous spans are based on the longest span. The shortest span shall not be less than $50 \%$ of the longest span.
2. The spans are based on uniform gravity loads only as listed for each table, including the effects of a 300 lb concentrated load. These spans have not been evaluated for wind
3. These tables do not reflect any additional stiffness provided by the roof sheathing.
4. Live load deflection is limited to $L / 240$.
5. Total load deflection is limited to $L / 180$.
6. The spans are based on an end bearing length of at least $1-3 / 4^{\prime \prime}$ and an interior bearing length of at least 3-1/2", and are limited to the bearing capacity for an SPF wall plate ( $\mathrm{F}_{\mathrm{c} \perp}=425 \mathrm{psi}$ ).

## ADDITIONAL NOTES:

1. Web stiffeners are not required for the Roof Span tables except when using a "bird's mouth" detail for the low-end bearing. Web fillers are required for I-Joists seated in hangers that do not laterally support the top flange or for hangers that require nailing into the web.
2. $\mathrm{L} / 240$ represents the maximum deflection allowed per code for roof joists supporting non-plaster ceilings. Verify deflection limits with local code requirements
3. Roof joists shall have a minimum pitch of $1 / 4^{\prime \prime}$ per foot ( $1 / 4: 12$ ) for positive drainage.
4. Roof applications in high wind areas require special analysis which may reduce spans and may require bracing of the bottom flange and special connectors to resist uplift.
5. For conditions not shown, use the Uniform Roof Load (PLF) tables, LP's design software or contact your LP ${ }^{\ominus}$ SolidStart ${ }^{\oplus}$ Engineered Wood Products distributor for assistance.


## ACTUAL DEFLECTION BASED ON SPAN AND LIMIT

| Span (ft) | L/360 | L/240 | L/180 |
| :---: | :---: | :---: | :---: |
| 10' | 5/16" | 1/2" | 11/16" |
| 12' | 3/8" | 5/8" | 13/16" |
| 14' | 7/16" | 11/16" | 15/16" |
| 16 ' | 9/16" | 13/16" | 1-1/16" |
| 18' | 5/8" | 7/8" | 1-3/16" |
| $20^{\prime}$ | 11/16" | 1" | 1-5/16" |
| 22' | 3/4" | 1-1/8" | 1-7/16" |
| 24' | 13/16" | 1-3/16" | 1-5/8" |
| 26' | 7/8" | 1-5/16" | 1-3/4" |
| 28' | 15/16" | 1-3/8" | 1-7/8" |
| $30^{\prime}$ | $1{ }^{1 \prime}$ | 1-1/2" | $2{ }^{\prime \prime}$ |

[^0]

## TO USE:

1. Select the appropriate set of tables based on roof pitch
2. Select the section of that table that corresponds to the design roof live load (snow or non-snow).
3. Find a span that meets or exceeds the design span for the appropriate roof dead load ( 15 psf or 20 psf ).
4. Read the corresponding series, depth and spacing

## DESIGN ASSUMPTIONS:

1. The spans listed are the horizontal clear distance between supports and are valid for simple or continuous span applications. Continuous spans are based on the longest span. The shortest span shall not be less than $50 \%$ of the longest span.
2. The spans are based on uniform gravity loads only as listed for each table, including the effects of a 300 lb concentrated load. These spans have not been evaluated for wind
3. These tables do not reflect any additional stiffness provided by the roof sheathing.
4. Live load deflection is limited to $L / 240$.
5. Total load deflection is limited to $\mathrm{L} / 180$.
6. The spans are based on an end bearing length of at least $1-3 / 4^{\prime \prime}$ and an interior bearing length of at least 3-1/2", and are limited to the bearing capacity for an SPF wall plate ( $\mathrm{F}_{\mathrm{c} \perp}=425 \mathrm{psi}$ ).

## ADDITIONAL NOTES:

1. Web stiffeners are not required for the Roof Span tables except when using a "bird's mouth" detail for the low-end bearing. Web fillers are required for I-Joists seated in hangers that do not laterally support the top flange or for hangers that require nailing into the web.
2. $\mathrm{L} / 240$ represents the maximum deflection allowed per code for roof joists supporting non-plaster ceilings. Verify deflection limits with local code requirements
3. Roof joists shall have a minimum pitch of $1 / 4$ " per foot $(1 / 4: 12)$ for positive drainage.
4. Roof applications in high wind areas require special analysis which may reduce spans and may require bracing of the bottom flange and special connectors to resist uplift.
5. For conditions not shown, use the Uniform Roof Load (PLF) tables, LP's design software or contact your LP ${ }^{\ominus}$ SolidStart ${ }^{\oplus}$ Engineered Wood Products distributor for assistance.


## ACTUAL DEFLECTION BASED ON SPAN AND LIMIT

| Span (ft) | L/360 | L/240 | L/180 |
| :---: | :---: | :---: | :---: |
| $10^{\prime}$ | 5/16" | $1 / 2^{\prime \prime}$ | 11/16" |
| 12' | 3/8" | 5/8" | 13/16" |
| $14{ }^{\prime}$ | 7/16" | 11/16" | 15/16" |
| $16^{\prime}$ | 9/16" | 13/16" | 1-1/16" |
| 18' | 5/8" | 7/8" | 1-3/16" |
| $20^{\prime}$ | 11/16" | $1{ }^{\prime \prime}$ | 1-5/16" |
| 22' | 3/4" | 1-1/8" | 1-7/16" |
| 24 | 13/16" | 1-3/16" | 1-5/8" |
| 26' | 7/8" | 1-5/16" | 1-3/4" |
| 28 | 15/16" | 1-3/8" | 1-7/8" |
| $30^{\prime}$ | $1^{\prime \prime}$ | 1-1/2" | $2{ }^{\prime \prime}$ |

[^1]

## TO USE:

1. Select the appropriate set of tables based on roof pitch
2. Select the section of that table that corresponds to the design roof live load (snow or non-snow).
3. Find a span that meets or exceeds the design span for the appropriate roof dead load ( 15 psf or 20 psf ).
4. Read the corresponding series, depth and spacing

## DESIGN ASSUMPTIONS:

1. The spans listed are the horizontal clear distance between supports and are valid for simple or continuous span applications. Continuous spans are based on the longest span. The shortest span shall not be less than $50 \%$ of the longest span.
2. The spans are based on uniform gravity loads only as listed for each table, including the effects of a 300 lb concentrated load. These spans have not been evaluated for wind
3. These tables do not reflect any additional stiffness provided by the roof sheathing.
4. Live load deflection is limited to $L / 240$.
5. Total load deflection is limited to $L / 180$.
6. The spans are based on an end bearing length of at least $1-3 / 4^{\prime \prime}$ and an interior bearing length of at least 3-1/2", and are limited to the bearing capacity for an SPF wall plate ( $\mathrm{F}_{\mathrm{c} \perp}=425 \mathrm{psi}$ ).

## ADDITIONAL NOTES:

1. Web stiffeners are not required for the Roof Span tables except when using a "bird's mouth" detail for the low-end bearing. Web fillers are required for I-Joists seated in hangers that do not laterally support the top flange or for hangers that require nailing into the web.
2. $\mathrm{L} / 240$ represents the maximum deflection allowed per code for roof joists supporting non-plaster ceilings. Verify deflection limits with local code requirements
3. Roof joists shall have a minimum pitch of $1 / 4^{\prime \prime}$ per foot $(1 / 4: 12)$ for positive drainage.
4. Roof applications in high wind areas require special analysis which may reduce spans and may require bracing of the bottom flange and special connectors to resist uplift.
5. For conditions not shown, use the Uniform Roof Load (PLF) tables, LP's design software or contact your LP ${ }^{\ominus}$ SolidStart ${ }^{\oplus}$ Engineered Wood Products distributor for assistance.


## ACTUAL DEFLECTION BASED ON SPAN AND LIMIT

| Span (ft) | L/360 | L/240 | L/180 |
| :---: | :---: | :---: | :---: |
| 10' | 5/16" | 1/2" | 11/16" |
| $12^{\prime}$ | 3/8" | 5/8" | 13/16" |
| $14{ }^{\prime}$ | 7/16" | 11/16" | 15/16" |
| $16^{\prime}$ | 9/16" | 13/16" | 1-1/16" |
| $18{ }^{\prime}$ | 5/8" | 7/8" | 1-3/16" |
| $20^{\prime}$ | 11/16" | $1{ }^{\prime \prime}$ | 1-5/16" |
| 22' | 3/4" | 1-1/8" | 1-7/16" |
| $24^{\prime}$ | 13/16" | 1-3/16" | 1-5/8" |
| $26^{\prime}$ | 7/8" | 1-5/16" | 1-3/4" |
| 28' | 15/16" | 1-3/8" | 1-7/8" |
| $30^{\prime}$ | $1^{\prime \prime}$ | 1-1/2" | $2{ }^{\prime \prime}$ |



[^2]
## TO USE:

1. Select the appropriate set of tables based on roof pitch
2. Select the section of that table that corresponds to the design roof live load (snow or non-snow)
3. Find a span that meets or exceeds the design span for the appropriate roof dead load (15 psf or 20 psf ).
4. Read the corresponding series, depth and spacing.

## DESIGN ASSUMPTIONS:

1. The spans listed are the horizontal clear distance between supports and are valid for simple or continuous span applications. Continuous spans are based on the longest span. The shortest span shall not be less than $50 \%$ of the longest span.
2. The spans are based on uniform gravity loads only as listed for each table, including the effects of a 300 lb concentrated load. These spans have not been evaluated for wind
3. These tables do not reflect any additional stiffness provided by the roof sheathing.
4. Live load deflection is limited to $L / 240$.
5. Total load deflection is limited to $L / 180$.
6. The spans are based on an end bearing length of at least $1-3 / 4^{\prime \prime}$ and an interior bearing length of at least $3-1 / 2$," and are limited to the bearing capacity for an SPF wall plate ( $\mathrm{F}_{\mathrm{c} \perp}=425 \mathrm{psi}$ ).

ADDITIONAL NOTES:

1. Web stiffeners are not required for the Roof Span tables except when using a "bird's mouth" detail for the low-end bearing. Web fillers are required for I-Joists seated in hangers that do not laterally support the top flange or for hangers that require nailing into the web.
2. $\mathrm{L} / 240$ represents the maximum deflection allowed per code for roof joists supporting non-plaster ceilings. Verify deflection limits with local code requirements
3. Roof joists shall have a minimum pitch of $1 / 4^{\prime \prime}$ per foot $(1 / 4: 12)$ for positive drainage.
4. Roof applications in high wind areas require special analysis which may reduce spans and may require bracing of the bottom flange and special connectors to resist uplift.
5. For conditions not shown, use the Uniform Roof Load (PLF) tables, LP’s design software or contact your LP® SolidStart ${ }^{\oplus}$ Engineered Wood Products distributor for assistance.


## ACTUAL DEFLECTION BASED ON SPAN AND LIMIT

| Span (ft) | L/360 | L/240 | L/180 |
| :---: | :---: | :---: | :---: |
| 10' | 5/16" | 1/2" | 11/16" |
| $12^{\prime}$ | 3/8" | 5/8" | 13/16" |
| $14{ }^{\prime}$ | 7/16" | 11/16" | 15/16" |
| $16^{\prime}$ | 9/16" | 13/16" | 1-1/16" |
| $18^{\prime}$ | 5/8" | 7/8" | 1-3/16" |
| $20^{\prime}$ | 11/16" | 1" | 1-5/16" |
| 22' | 3/4" | 1-1/8" | 1-7/16" |
| $24^{\prime}$ | 13/16" | 1-3/16" | 1-5/8" |
| $26^{\prime}$ | 7/8" | 1-5/16" | 1-3/4" |
| 28' | 15/16" | 1-3/8" | 1-7/8" |
| $30^{\prime}$ | $1^{\prime \prime}$ | 1-1/2" | $2 "$ |



[^3]
## TO USE:

1. Select the required product.
2. Follow across the row to the required joist spacing
3. Select the row corresponding to the Roof Load and Truss Span needed. 4. The letter represents the required detail.



DESIGN ASSUMPTIONS:

1. These tables are valid for joists selected from the Floor Span Tables in this design guide.
2. The floor design loads shall not exceed 40 psf Live and 25 psf Dead Load
3. The maximum cantilever length is $2^{\prime}$ (the amount of overhang).
4. The wall weight is assumed to be 100 plf.
5. Roof Load is the Total Load (Live plus Dead) on the roof trusses.
6. Truss Span is the out-to-out distance of the truss bearing walls as shown above.
7. A maximum overhang of $2^{\prime}$ is allowed for the trusses.
8. These tables assume a cantilever-end bearing length of at least $3-1 / 2^{\prime \prime}$ and are limited to the bearing capacity for an SPF wall plate ( $\mathrm{F}_{\mathrm{c} \perp}=425 \mathrm{psi}$ )

## ADDITIONAL NOTES:

1. The reinforcement (when required) shall match the depth of the I-Joist.
2. The closure at the end of the cantilever shall be a minimum of $23 / 32^{\prime \prime}$ APA Rated OSB (or equal). Certain codes may require a thicker product for lateral load transfer.
3. For short cantilevers up to 6 " long, use the Brick-Ledge Cantilever Tables on the following pages.
4. For conditions not shown, use LP's design software or contact your LP SolidStart Engineered Wood Products distributor for assistance.
5. Do not use a product where designated "-" without further analysis by a design professional

## TO USE:

1. Select the required product.
2. Follow across the row to the required joist spacing
3. Select the row corresponding to the Roof Load and Truss Span needed.
4. The letter represents the required detail.



## DESIGN ASSUMPTIONS:

1. These tables are valid for joists selected from the Floor Span Tables in this design guide.
2. The floor design loads shall not exceed 40 psf Live and 25 psf Dead Load.
3. The maximum cantilever length is 2 ' (the amount of overhang).
4. The wall weight is assumed to be 100 plf.
5. Roof Load is the Total Load (Live plus Dead) on the roof trusses.
6. Truss Span is the out-to-out distance of the truss bearing walls as shown above.
7. A maximum overhang of $2^{\prime}$ is allowed for the trusses.
8. These tables assume a cantilever-end bearing length of at least $3-1 / 2^{\prime \prime}$ and are limited to the bearing capacity for an SPF wall plate ( $\mathrm{F}_{\mathrm{c} \perp}=425 \mathrm{psi}$ ).

## ADDITIONAL NOTES:

1. The reinforcement (when required) shall match the depth of the I-Joist.
2. The closure at the end of the cantilever shall be a minimum of $23 / 32^{\prime \prime}$ APA Rated OSB (or equal). Certain codes may require a thicker product for lateral load transfer.
3. For short cantilevers up to 6 " long, use the Brick-Ledge Cantilever Tables on the following pages.
4. For conditions not shown, use LP's design software or contact your LP SolidStart Engineered Wood Products distributor for assistance.
5. Do not use a product where designated "-" without further analysis by a design professional

TO USE:

1. Select the required product.
2. Select the row corresponding to the Roof Load and Truss Span needed.
3. Follow across the row to the required joist spacing
4. The letter represents the required detail.
las


## DESIGN ASSUMPTIONS:

1. These tables are valid for joists selected from the Floor Span Tables in this design guide.
2. The floor design loads shall not exceed 40 psf Live and 25 psf Dead Load
3. The maximum cantilever length is 2 ' (the amount of overhang).
4. The wall weight is assumed to be 100 plf.
5. Roof Load is the Total Load (Live plus Dead) on the roof trusses.
6. Truss Span is the out-to-out distance of the truss bearing walls as shown above.
7. A maximum overhang of $2^{\prime}$ is allowed for the trusses.
8. These tables assume a cantilever-end bearing length of at least $3-1 / 2^{\prime \prime}$ and are limited to the bearing capacity for an SPF wall plate ( $\mathrm{F}_{\mathrm{c} \perp}=425 \mathrm{psi}$ ).

## ADDITIONAL NOTES:

1. The reinforcement (when required) shall match the depth of the I-Joist
2. The closure at the end of the cantilever shall be a minimum of $23 / 32$ " APA Rated OSB (or equal) Certain codes may require a thicker product for lateral load transfer.
3. For short cantilevers up to 6 " long, use the Brick-Ledge Cantilever Tables on the following pages.
4. For conditions not shown, use LP's design software or contact your LP SolidStart Engineered Wood Products distributor for assistance.
5. Do not use a product where designated "-" without further analysis by a design professional

## TO USE:

1. Select the required product.
2. Follow across the row to the required joist spacing
3. Select the row corresponding to the Roof Load and Truss Span needed. 4. The letter represents the required detail.

## 23/32" OSB (OR EQUAL) REINFORCEMENT BOTH SIDES



| Roof Load | $\begin{gathered} \text { Truss } \\ \text { Span } \\ (\mathrm{ft}) \end{gathered}$ | 16" LPI 20Plus |  |  |  | 16" LPI 32Plus |  |  |  | 16" LPI 36 |  |  |  | 16" LPI 42Plus |  |  |  | 16" LPI 52Plus |  |  |  | 16" LPI 56 |  |  |  | $\begin{array}{\|c\|} \hline \text { Truss } \\ \text { Span } \\ \text { (ft) } \\ \hline \end{array}$ | Roof Load |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 12" | 16" | 19.2" | 24" | 12" | 16" | 19.2" | 24" | 12" | 16" | 19.2" | 24" | 12" | 16" | 19.2" | 24" | 12" | 16" | 19.2" | 24" | 12" | 16" | 19.2" | 24" |  |  |  |
| $\begin{aligned} & 30 \\ & \text { psf } \end{aligned}$ | 22' | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | $22^{\prime}$ | $\begin{aligned} & 30 \\ & \text { psf } \end{aligned}$ |  |
|  | 24' | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | $24^{\prime}$ |  |  |
|  | $26^{\prime}$ | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | 26' |  |  |
|  | 28' | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C2 | C1 | C1 | C1 | C1 | $\mathrm{Cl}_{1}$ | C1 | C1 | C1 | C1 | C1 | C1 | C1W | 28' |  |  |
|  | $30^{\prime}$ | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C2 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1W | 30' |  |  |
|  | 32' | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C1W | C1 | C1 | C1W | C2 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1W | 32' |  |  |
|  | 34' | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C2 | C1 | C1 | C1w | C2 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1W | 34' |  |  |
|  | 36' | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C2 | C1 | C1 | C1W | C2 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1W | 36' |  |  |
|  | 38' | C1 | C1 | C1W | C2 | C1 | C1 | c1w | C2 | C1 | C1 | C1W | c2 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1W | 38' |  |  |
|  | 40' | C1 | C1 | C1W | C2 | C1 | C1 | C1W | c2 | C1 | C1 | C1W | C2 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1W | 40' |  |  |
|  | 42' | C1 | C1 | C1W | C2 | C1 | C1 | c1w | C2 | C1 | C1 | C1w | C2 | C1 | C1 | C1 | c1w | C1 | C1 | C1 | C1 | $C_{1}$ | C1 | C1 | C1W | 42' |  |  |
| $\begin{aligned} & 40 \\ & \text { psf } \end{aligned}$ | $22^{\prime}$ | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C1W | C1 | C1 | C1W | C2 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1W | $22^{\prime}$ | $\begin{aligned} & 40 \\ & \text { psf } \end{aligned}$ | N |
|  | 24' | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C1 | C1 | C1 | C1W | c2 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1W | $24^{\prime}$ |  |  |
|  | $26^{\prime}$ | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C2 | C1 | C1 | C1w | C2 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1W | $26^{\prime}$ |  |  |
|  | 28' | C1 | C1 | C1W | C2 | C1 | C1 | c1w | C2 | C1 | C1 | C1w | C2 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1W | 28' |  |  |
|  | 30' | C1 | C1 | C1W | C2 | C1 | C1 | c1w | C2 | C1 | C1 | C1W | C2 | C1 | C1 | C1 | C1 | $\mathrm{Cl}_{1}$ | C1 | C1 | C1 | C1 | C1 | C1 | C1W | $30^{\prime}$ |  |  |
|  | 32' | C1 | C1 | C1w | C2 | C1 | C1 | c1w | C2 | C1 | C1 | C2 | C3 | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1W | 32' |  |  |
|  | 34' | C1 | C1W | C1w | C2 | C1 | C1W | c1w | C2 | C1 | C1 | C2 | С3 | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C1W | 34' |  |  |
|  | 36' | C1 | C1w | C1W | C3 | C1 | C1W | c1w | С3 | C1 | C1W | C2 | С3 | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C1W | 36' |  |  |
|  | 38' | C1 | C1W | C1w | - | C1 | C1W | C2 | - | C1 | C1W | C2 | C3 | C1 | C1 | C1 | C1W | C1 | C1 | C1 | c1w | C1 | C1 | C1 | C1W | 38' |  |  |
|  | 40' | C1 | C1W | C1W | - | C1 | C1W | C2 |  | C1 | C1W | C2 | С3 | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C1W | 40' |  |  |
|  | 42' | C1 | C1W | C2 |  | C1 | C1W | C2 |  | C1 | C1W | C2 | С3 | C1 | C1 | C1w | C1W | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C1W | 42' |  |  |
| $\begin{aligned} & 50 \\ & \text { psf } \end{aligned}$ | $22^{\prime}$ | C1 | C1 | C1W | C2 | C1 | C1 | C1W | C2 | C1 | C1 | C1W | C2 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1W | 22' | $\begin{aligned} & 50 \\ & \text { psf } \end{aligned}$ | + |
|  | 24' | C1 | C1 | C1W | C2 | C1 | C1 | C1w | c2 | C1 | C1 | C1W | C2 | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C1 | C1 | C1 | C1 | C1W | $24^{\prime}$ |  | - |
|  | $26^{\prime}$ | C1 | C1W | C1W | C2 | C1 | C1 | C1W | C2 | C1 | C1 | C2 | C3 | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C1W | $26^{\prime}$ |  | \% |
|  | 28' | C1 | C1W | C1W | C3 | C1 | C1W | c1w | C3 | C1 | C1W | C2 | С3 | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C1W | 28' |  | T |
|  | 30 | C1 | C1W | C1W | - | C1 | C1W | C2 | - | C1 | C1W | C2 | C3 | C1 | C1 | C1 | C1W | C1 | C1 | C1 | c1w | C1 | C1 | C1W | C1W | 30' |  |  |
|  | 32' | $\mathrm{Cl}_{1}$ | C1w | C1w | - | C1 | C1W | C2 | - | C1 | C1W | C2 | C3 | C1 | C1 | C1w | C1w | C1 | C1 | C1 | c1w | C1 | C1 | C1W | C1W | 32' |  |  |
|  | 34' | C1 | C1W | C2 | - | C1 | C1W | C2 | - | C1 | C1W | C2 | - | C1 | C1 | C1w | C1W | C1 | C1 | C1W | C1W | C1 | C1 | C1W | C1W | 34' |  |  |
|  | 36' | C1 | C1w | C2 | - | C1 | C1W | C2 | - | C1 | C1W | C2 | - | C1 | C1 | C1W | С3 | C1 | C1 | C1W | C1W | C1 | C1W | C1W | C1W | 36' |  |  |
|  | 38' | C1W | C1w | С3 | - | C1W | C1W | C3 | - | C1 | C2 | С3 | - | C1 | C1 | C1W | C3 | C1 | C1 | C1w | C1W | C1 | C1W | C1W | C1W | 38' |  |  |
|  | 40' | C1W | C1w | - | - | C1W | C1W | - | - | C1 | C2 | С3 | - | C1 | C1W | C1W | С3 | C1 | C1 | C1W | C1W | C1 | C1W | C1W | - | $40^{\prime}$ |  |  |
|  | 42' | C1W | C1W | - | - | C1W | c2 | - | - | C1 | C2 | С3 | - | C1 | C1W | C1W | С3 | C1 | C1 | C1W | c3 | C1 | C1w | C1W | - | 42' |  |  |
| $\begin{aligned} & 60 \\ & \text { psf } \end{aligned}$ | 22' | C1 | C1W | C1W | C2 | C1 | C1W | c1w | C2 | C1 | C1 | C2 | C3 | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C1W | 22' | $\begin{aligned} & 60 \\ & \text { psf } \end{aligned}$ |  |
|  | 24 | C1 | C1W | C1W | - | C1 | C1W | C2 | - | C1 | C1W | C2 | C3 | C1 | C1 | C1 | C1W | C1 | C1 | C1 | C1W | C1 | C1 | C1W | C1W | $24^{\prime}$ |  |  |
|  | $26^{\prime}$ | $\mathrm{Cl}_{1}$ | C1W | C1w | - | C1 | C1W | C2 | - | C1 | C1W | C2 | C3 | $\mathrm{C}_{1}$ | C1 | C1w | C1w | $\mathrm{Cl}_{1}$ | C1 | C1 | c1w | C1 | C1 | C1W | C1W | $26^{\prime}$ |  |  |
|  | 28' | C1 | C1w | c2 | - | C1 | C1W | C2 | - | C1 | C1W | C2 | - | C1 | C1 | C1w | С3 | C1 | C1 | C1W | C1W | C1 | C1 | C1W | C1W | 28' |  |  |
|  | 30' | C1 | C1W | C3 | - | C1 | C1W | C3 | - | C1 | C1W | C2 | - | C1 | C1 | C1W | С3 | C1 | C1 | C1W | C1W | C1 | C1W | C1W | C1W | 30' |  |  |
|  | 32' | C1W | C1W | - | - | C1W | C1W | - | - | C1 | C2 | C3 | - | C1 | C1 | C1W | C3 | C1 | C1 | C1W | C1W | C1 | C1W | C1W | C3 | 32' |  |  |
|  | 34' | c1w | C1w | - | - | C1W | C1W | - | - | C1 | C2 | C3 | - | C1 | C1W | C1w | - | C1 | C1 | C1W | C1W | C1 | c1w | C1W | - | 34' |  |  |
|  | 36' | C1W | C1W | - | - | C1W | C2 | - | - | C1W | C2 | С3 | - | C1 | C1W | C1W | - | C1 | C1W | C1W | C3 | C1 | C1W | C1W | - | 36' |  |  |
|  | 38' | c1w | C3 | - | - | C1w | C3 | - | - | C1W | C2 | - | - | C1 | C1W | C3 | - | C1 | C1W | C1w | - | C1 | c1w | C1W | - | 38' |  |  |
|  | 40' | C1w | - | - | - | C1W | - | - | - | C1W | C2 | - | - | C1 | C1W | C3 | - | C1 | C1W | C1W | - | C1 | c1w | C1W | - | $40^{\prime}$ |  |  |
|  | 42' | c1w | - | - | - | C1W | - | - | - | C1W | C3 | - | - | C1 | C1W | C3 | - | $\mathrm{C1}_{1}$ | C1W | C1W |  | c1w | C1W | C1W |  | $42^{\prime}$ |  |  |

## DESIGN ASSUMPTIONS:

1. These tables are valid for joists selected from the Floor Span Tables in this design guide.
2. The floor design loads shall not exceed 40 psf Live and 25 psf Dead Load.
3. The maximum cantilever length is 2 ' (the amount of overhang).
4. The wall weight is assumed to be 100 plf.
5. Roof Load is the Total Load (Live plus Dead) on the roof trusses.
6. Truss Span is the out-to-out distance of the truss bearing walls as shown above.
7. A maximum overhang of $2^{\prime}$ is allowed for the trusses.
8. These tables assume a cantilever-end bearing length of at least $3-1 / 2^{\prime \prime}$ and are limited to the bearing capacity for an SPF wall plate ( $\mathrm{F}_{\mathrm{c} \perp}=425 \mathrm{psi}$ ).

## ADDITIONAL NOTES:

1. The reinforcement (when required) shall match the depth of the I-Joist.
2. The closure at the end of the cantilever shall be a minimum of $23 / 32^{\prime \prime}$ APA Rated OSB (or equal). Certain codes may require a thicker product for lateral load transfer.
3. For short cantilevers up to 6 " long, use the Brick-Ledge Cantilever Tables on the following pages.
4. For conditions not shown, use LP's design software or contact your LP SolidStart Engineered Wood Products distributor for assistance.
5. Do not use a product where designated "-" without further analysis by a design professional.

## TOTAL JOIST REACTION CALCULATION

LP® SolidStart ${ }^{\oplus}$ I-Joists can cantilever up to 6 " to support a load-bearing wall over a brick finish. Depending on the Total Joist Reaction (TJR), the joists may require reinforcement. If the TJR is less than the End Reaction Capacity W/out Stiffeners (page 4), then no reinforcement is required. If the TJR is greater than the End Reaction Capacity W/out Stiffeners, but less than the End Reaction Capacity With Stiffeners, then web stiffeners shall be installed at the bearing. Otherwise, one of the reinforcing details from below shall be used.

TOTAL JOIST REACTION, TJR = FLR + WLR + RLR
Where: FLR = Floor Load Reaction
WLR = Wall Load Reaction
RLR $=$ Roof Load Reaction, including any other floor, ceiling or attic loads imposed on wall


BRICK LEDGE CANTILEVER FULL-DEPTH REINFORCING One Side


* NOTE: Pilot holes required when using screws

| Series | Minimum Web Filler | Reaction Capacity <br> (Ibs) |
| :---: | :---: | :---: |
| LPI 18 | $23 / 32^{\prime \prime}$ APA Rated OSB (or equal) | 2300 |
| LPI 20Plus <br> LPI 32Plus <br> LPI 36 | $23 / 32^{\prime \prime}$ APA Rated OSB (or equal) | 2600 |
| LPI 42Plus <br> LPI 52Plus <br> LPI 56 | 1-1/8" APA Rated OSB (or equal) | 4000 |


| Series | Reaction Capacity (Ibs) |  |
| :---: | :---: | :---: |
|  | 23/32" APA Rated OSB <br> (or equal) | 1" Min. <br> LP SolidStart Rim Board |
| LPI 18 | 3100 | 3400 |
| LPI 20Plus <br> LPI 32Plus <br> LPI 36 | 3500 | 3800 |
| LPI 42Plus <br> LPI 52Plus <br> LPI 56 | 4800 | 5200 |

EXAMPLE 1:


EXAMPLE 2:


[^4]
## Web Hole Specifications: Circular Holes



TO USE:

1. Select the required series and depth.
2. Determine the support condition for the nearest bearing: end support or interior support (including cantilever-end supports).
3. Select the row corresponding to the required Clear Span. For spans between those listed, use the next largest value.

4 Select the column corresponding to the required hole diameter. For diameters between those listed, use the next largest value.
5. The intersection of the Clear Span row and Hole Diameter column gives the minimum distance from the inside face of bearing to the center of a circular hole
6. Double check the distance to the other support, using the appropriate support condition.

| Series | Depth | Clear Span (ft) | Distance from End Support |  |  |  |  |  | Distance from Interior or Cantilever-End Support |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Hole Diameter |  |  |  |  |  | Hole Diameter |  |  |  |  |  |
|  |  |  | 2" | 4" | 6" | 8" | 10" | 12" | 2" | 4" | $6{ }^{\prime \prime}$ | 8" | 10" | 12" |
| LPI 18 | 9-1/2" | $6{ }^{1}$ | 1'-0" | 1'-0" | 1'-0" | - | - | - | 1'-0" | 1'-0" | 1'-0" | - | - | - |
|  |  | $10^{\prime}$ | 1'-0" | 1'-0" | 2'-1' | - | - | - | $1^{\prime}-0{ }^{\prime \prime}$ | 1'-3" | $3^{\prime}-1{ }^{\prime \prime}$ | - | - | - |
|  |  | $14{ }^{\prime}$ | 1'-0" | 2'-2" | 4'-6" | - | - | - | 1'-11" | 3'-9" | 5'-7" | - | - | - |
|  |  | 18' | 2'-4" | 4'-7" | 7'-2' | - | - | - | 4'-5" | 6'-3" | $8^{\prime}-4{ }^{\prime \prime}$ | - | - | - |
|  | 11-7/8" | $10^{\prime}$ | 1'-0" | 1'-0" | 1'-0" | 1'-10" | - | - | 1'-0" | 1'-0" | 1'-3" | 3'-0" | - | - |
|  |  | $14^{\prime}$ | 1'-0" | 1'-0" | 2'-1' | 4'-4" | - | - | 1'-0" | 2'-0" | 3'-9" | 5'-6" | - | - |
|  |  | 18' | 1'-0" | 2'-5" | 4'-6" | 6'-11" | - | - | 2'-9" | 4'-6" | 6'-3" | 8'-1" | - | - |
|  |  | 22' | 2'-8" | 4'-9" | 7'-0" | 9'-8" | - | - | 5'-3' | 7'-0" | 8'-9" | 11'-0" | - | - |
|  | 14" | $14^{\prime}$ | 1'-0" | 1'-0" | 1'-0" | 2'-3' | 4'-5" | - | 1'-0" | 1'-0" | 2'-2" | 3'-10" | 5'-6" | - |
|  |  | 18' | 1'-0" | 1'-0" | 2'-7" | 4'-8" | 7'-0" | - | 1'-4" | 3'-0" | 4'-8" | $6^{\prime}-4{ }^{\prime \prime}$ | 8'-2' | - |
|  |  | 22' | $1^{\prime}-1{ }^{\prime \prime}$ | 2'-11" | 4'-11" | 7'-2' | 9'-9" | - | 3'-10" | 5'-6" | 7'-2' | 8'-10" | - | - |
|  |  | $26^{\prime}$ | $3^{\prime}-3^{\prime \prime}$ | 5'-3" | 7'-5" | 9'-9" | 12'-6" | - | $6^{\prime}-4{ }^{\prime \prime}$ | 8'-0" | 9'-8" | 11'-6" | - | - |
|  <br> LPI 32Plus | 9-1/2" | $6{ }^{\prime}$ | 1'-0" | 1'-0" | 1'-0" | - | - | - | 1'-0" | 1'-0" | 1'-0" | - | - | - |
|  |  | $10^{\prime}$ | 1'-0" | 1'-0" | 1'-0" | - | - | - | $1^{\prime}-0{ }^{\prime \prime}$ | 1'-0" | 1'-0" | - | - | - |
|  |  | 14 | 1'-0" | 1'-0" | 1'-5" | - | - | - | 1'-0" | 1'-5" | 3'-1' | - | - | - |
|  |  | 18' | 1'-0" | 1'-9" | 3'-8" | - | - | - | 2'-3' | 3'-11" | 5'-7" | - | - | - |
|  | 11-7/8" | $10^{\prime}$ | 1'-0" | 1'-0" | 1'-0" | 1'-0" | - | - | 1'-0" | 1'-0" | 1'-0" | 1'-0" | - | - |
|  |  | $14^{\prime}$ | 1'-0" | 1'-0" | 1'-0" | 1'-9" | - | - | 1'-0" | 1'-0" | 2'-1' | 3'-5" | - | - |
|  |  | 18' | 1'-0" | 1'-0" | 2'-6" | 4'-1" | - | - | 1'-10" | 3'-3" | 4'-7" | 5'-11" | - | - |
|  |  | 22' | 1'-8" | 3'-2" | 4'-10" | 6'-7" | - | - | $4^{\prime}-4^{\prime \prime}$ | 5'-9" | $7^{\prime}-1{ }^{\prime \prime}$ | 8'-5" | - | - |
|  | 14" | $14{ }^{\prime}$ | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 2'-2" | - | 1'-0" | 1'-0" | 1'-5" | 2'-7" | 3'-9" | - |
|  |  | 18' | 1'-0" | 1'-0" | 1'-9" | 3'-1' | 4'-6" | - | 1'-8" | 2'-10" | 3'-11" | 5'-1" | 6'-3" | - |
|  |  | 22' | 1'-5" | 2'-9" | 4'-1" | 5'-6" | 7'-0" | - | 4'-2" | 5'-4" | 6'-5" | 7'-7" | 8'-9" | - |
|  |  | $26^{\prime}$ | $3^{\prime}-8{ }^{\prime \prime}$ | 5'-0" | 6'-5' | 8'-0' | 9'-8" | - | 6'-8' | 7'-10" | 8'-11" | 10'-1" | 11'-4" | - |
|  | 16" | 18' | 1'-0" | 1'-0" | 1'-4" | 2'-5' | 3'-7" | 4'-11" | 1'-6" | 2'-6" | 3'-6" | 4'-6" | 5'-6" | 6'-6" |
|  |  | 22' | 1'-4" | 2'-5" | 3'-6" | 4'-9" | $6^{\prime}-1{ }^{\prime \prime}$ | 7'-5" | 4'-0' | 5'-0" | 6'-0" | 7'-0" | 8'-0' | 9'-0" |
|  |  | $26^{\prime}$ | 3'-6" | 4'-8" | 5'-11" | 7'-2" | 8'-7" | 10'-1" | 6'-6" | 7'-6" | 8'-6" | 9'-6" | 10'-6" | 11-9" |
|  |  | $30^{\prime}$ | 5'-9" | 7'-0" | 8'-4" | 9'-9" | 11'-3" | 12'-10" | 9'-0" | 10'-0" | 11'-0" | 12'-0" | 13'-2" | 14'-8" |
| $\begin{gathered} \text { LPI } 36 \\ \text { G } \\ \text { LPI } 56 \end{gathered}$ | 11-7/8" | $10^{\prime}$ | 1'-0" | 1'-0" | 1'-0" | 1'-0" | - | - | 1'-0" | 1'-0" | 1'-0" | 1'-3' | - | - |
|  |  | $14^{\prime}$ | 1'-0" | 1'-0" | 1'-0" | 2'-2' | - | - | 1'-0" | 1'-0" | 1'-8" | 3'-9" | - | - |
|  |  | 18' | 1'-0" | 1'-0" | 2'-0" | 4'-7" | - | - | 1'-0" | 2'-1" | 4'-2" | 6'-3" | - | - |
|  |  | 22' | 1'-0" | 1'-11' | 4'-4" | 7'-1' | - | - | 2'-6" | 4'-7" | 6'-8" | 8'-9" | - | - |
|  | 14" | $14^{\prime}$ | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 2'-10" | - | 1'-0" | 1'-0" | 1'-0" | 2'-6" | 4'-4" | - |
|  |  | 18' | 1'-0" | 1'-0" | 1'-0" | $3^{\prime}-0^{\prime \prime}$ | 5'-3' | - | 1'-0" | $1^{\prime}-5^{\prime \prime}$ | 3'-3' | 5'-0" | 6'-10" | - |
|  |  | 22' | 1'-0" | 1'-3" | 3'-2" | 5'-4" | 7'-10" | - | 2'-2' | 3'-11" | 5'-9" | 7'-6" | 9'-4" | - |
|  |  | $26^{\prime}$ | 1'-5" | $3^{\prime}-5^{\prime \prime}$ | 5'-6" | 7'-10" | 10'-6" | - | 4'-8" | $6^{\prime}-5^{\prime \prime}$ | $8^{\prime}-3^{\prime \prime}$ | 10'-0' | 12'-2" | - |
|  | 16" | 18' | 1'-0" | 1'-0" | 1'-0" | 2'-0" | 3'-10" | 5'-11" | 1'-0" | 1'-0" | 2'-7" | 4'-1" | 5'-8" | 7'-3' |
|  |  | $22^{\prime}$ | 1'-0" | 1'-0" | 2'-5" | 4'-3" | $6^{\prime}-3{ }^{\prime \prime}$ | 8'-6" | $1^{\prime}-11^{\prime \prime}$ | 3'-6" | 5'-1" | $6^{\prime}-7{ }^{\prime \prime}$ | 8'-2' | 9'-11" |
|  |  | $26^{\prime}$ | 1'-3" | 2'-11" | 4'-8" | 6'-8" | 8'-10" | 11'-3" | 4'-5" | 6'-0" | 7'-7" | 9'-1" | 10'-8" | 12'-10" |
|  |  | $30^{\prime}$ | 3'-4" | 5'-2" | 7'-1' | 9'-2" | 11'-5" | 14'-0" | 6'-11" | 8'-6" | 10'-1" | 11'-7" | 13'-5" | - |
| $\begin{aligned} & \text { LPI 42Plus } \\ & \text { G } \\ & \text { LPI } 52 \text { Plus } \end{aligned}$ | 9-1/2" | $6{ }^{\prime}$ | 1'-0" | 1'-0" | 1'-0" | - | - | - | 1'-0" | 1'-0" | 1'-0" | - | - | - |
|  |  | 10' | 1'-0" | 1'-0" | 1'-0" | - | - | - | $1^{1}-0^{\prime \prime}$ | 1'-0" | $1^{\prime}-0^{\prime \prime}$ | - | - | - |
|  |  | $14^{\prime}$ | 1'-0" | 1'-0" | 1'-5" | - | - | - | 1'-0" | 1'-5" | 3'-1' | - | - | - |
|  |  | 18' | 1'-0" | 1'-9" | 3'-8" | - | - | - | $2^{\prime}-3^{\prime \prime}$ | $3^{\prime}-11^{\prime \prime}$ | 5'-7" | - | - | - |
|  | 11-7/8" | 10' | 1'-0" | 1'-0" | 1'-0" | 1'-0" | - | - | 1'-0" | 1'-0" | 1'-0" | 1'-0" | - | - |
|  |  | $14^{\prime}$ | 1'-0" | 1'-0" | 1'-0" | 1'-9" | - | - | $1^{1}-0^{\prime \prime}$ | 1'-0" | $2^{\prime}-1$ ' | 3'-5" | - | - |
|  |  | 18' | 1'-0" | 1'-0" | 2'-6" | 4'-1" | - | - | 1'-10" | 3'-3" | 4'-7" | 5'-11" | - | - |
|  |  | 22' | 1'-8" | 3'-2' | 4'-10" | $6^{\prime}-7{ }^{\prime \prime}$ | - | - | $4^{\prime}-4{ }^{\prime \prime}$ | 5'-9" | $7^{\prime}-1{ }^{\prime \prime}$ | 8'-5" | - | - |
|  | 14" | $14^{\prime}$ | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 2'-2" | - | 1'-0" | 1'-0" | 1'-5" | 2'-7' | 3'-9" | - |
|  |  | 18' | 1'-0" | 1'-0" | 1'-9" | $3^{\prime}-1{ }^{\prime \prime}$ | 4'-6" | - | 1'-8" | 2'-10" | 3'-11" | 5'-1" | 6'-3" | - |
|  |  | 22' | 1'-5" | 2'-9" | 4'-1" | 5'-6" | 7'-0" | - | 4'-2" | 5'-4' | 6'-5" | 7'-7" | 8'-9" | - |
|  |  | $26^{\prime}$ | 3'-8" | 5'-0" | 6'-5" | 8'-0" | 9'-8" | - | 6'-8' | 7'-10" | 8'-11" | 10'-1" | 11'-4" | - |
|  | 16" | 18' | 1'-0" | 1'-0" | 1'-4" | 2'-5" | 3'-7" | 4'-11" | 1'-6" | 2'-6" | 3'-6" | 4'-6" | 5'-6" | 6'-6" |
|  |  | $22^{\prime}$ | 1'-4" | 2'-5" | 3'-6" | 4'-9" | $6^{\prime}-1{ }^{\prime \prime}$ | 7'-5" | 4'-0' | 5'-0" | $6^{\prime}-0^{\prime \prime}$ | 7'-0" | 8'-0" | 9'-0" |
|  |  | $26^{\prime}$ | 3'-6" | 4'-8" | 5'-11" | 7'-2" | 8'-7" | 10'-1" | 6'-6" | 7'-6" | 8'-6" | 9'-6" | 10'-6" | 11-9" |
|  |  | $30^{\prime}$ | 5'-9" | 7'-0" | 8'-4" | 9'-9" | 11'-3" | 12'-10" | 9'-0" | 10'-0" | 11'-0" | 12'-0" | 13'-2" | 14'-8" |

## DESIGN ASSUMPTIONS:

1. The hole locations listed above are valid for floor joists supporting only uniform loads. The total uniform load shall not exceed 130 plf (e.g., 40 psf Live Load and 25 psf Dead Load spaced $244^{\prime \prime}$ oc).
2. Hole location is measured from the inside face of bearing to the center of a circular hole, from the closest support.
3. Clear Span has not been verified for these joists and is shown for informational purposes only! Verify that the joist selected will work for the span and loading conditions needed before checking hole location.
4. The maximum hole depth for circular holes is the I-joist Depth less 4", except the maximum hole depth is $6^{\prime \prime}$ for $9-1 / 2^{1 "}$ LPl joists, and $8^{\prime \prime}$ for 11-7/8 LPI joists.
5. Holes cannot be located in the span where designated "-", without further analysis by a design professional

NOTES:

1. Holes may be placed anywhere within the depth of the joist. A minimum $1 / 4^{\prime \prime}$ clear distance is required between the hole and the flanges.
2. Round holes up to $1-1 / 2^{\prime \prime}$ diameter may be placed anywhere in the web.
3. Perforated "knockouts" may be neglected when locating web holes
4. Holes larger than $1-1 / 2^{\prime \prime}$ are not permitted in cantilevers without special engineering.
5. Multiple holes shall have a clear separation along the length of the joist of at least twice the length of the larger adjacent hole, or a minimum of 12" center-to-center, whichever is greater.
6. Multiple holes may be spaced closer provided they fit within the boundary of an acceptable larger hole. Example: two 3" round holes aligned parallel to the joist length may be spaced 2" apart (clear distance) provided that a $3^{\prime \prime}$ high by 8 " long rectangle or an 8 " diameter round hole are acceptable for the joist depth at that location and completely encompass the holes.
7. For conditions not covered in this table, us LP's design software or contact your local LP® SolidStart ${ }^{\oplus}$ Engineered Wood Products distributor for more information.


TO USE:
1 Select the required series and depth.
2. Determine the support condition for the nearest bearing: end support or interior support (including cantilever-end supports).
3. Select the row corresponding to the required Clear Span. For spans between those listed, use the next largest value.

4 Select the column corresponding to the required hole dimension. For dimensions between those listed, use the next largest value
5. The intersection of the Clear Span row and Hole Dimension column gives the minimum distance from the inside face of bearing to the nearest edge of a square or rectangular hole
6. Double check the distance to the other support, using the appropriate support condition.

| Series | Depth | Clear <br> Span <br> (ft) | Distance from End Support |  |  |  |  |  | Distance from Interior or Cantilever-End Support |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Maximum Hole Dimension: Depth or Width |  |  |  |  |  | Maximum Hole Dimension: Depth or Width |  |  |  |  |  |
|  |  |  | 2" | 4" | $6{ }^{\prime \prime}$ | 8" | 10" | 12" | 2" | 4" | $6{ }^{\prime \prime}$ | 8" | 10" | 12" |
| LPI 18 | 9-1/2" | $6{ }^{1}$ | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 1'-2" | 1'-7" | 1'-0" | 1'-0" | 1'-3" | 1'-6" | 1'-10" | 2'-2" |
|  |  | $10^{\prime}$ | 1'-0" | 1'-4" | 2'-10" | $3^{\prime}-3^{\prime \prime}$ | 3'-9" | 4'-3" | 1'-3" | 2'-6" | 3'-9' | 4'-0" | 4'-5" | - |
|  |  | $14{ }^{\prime}$ | 2'-2' | 3'-8" | 5'-5" | 5'-11" | 6'-6" | - | 3'-9" | 5'-0" | 6'-4" | - | - | - |
|  |  | 18' | $4^{\prime}-7{ }^{\prime \prime}$ | 6'-3' | 8'-2' | - | - | - | 6'-3' | 7'-6" | - | - | - | - |
|  | 11-7/8" | $10^{\prime}$ | 1'-0" | 1'-0" | 2'-2" | 3'-6" | 4'-0" | - | 1'-1" | 2'-2' | 3'-2" | 4'-2" | - | - |
|  |  | $14^{\prime}$ | 2'-0" | $3^{\prime}-3^{\prime \prime}$ | 4'-8" | 6'-3' | - | - | 3'-7" | 4'-8" | 5'-8" | - | - | - |
|  |  | $18^{\prime}$ | 4'-4" | 5'-9" | 7'-3' | - | - | - | 6'-1" | 7'-2' | 8'-5" | - | - | - |
|  |  | 22' | 6'-10" | 8'-4" | 10'-1" | - | - | - | 8'-7" | 9'-9" |  | - | - | - |
|  | 14" | $14^{\prime}$ | 1'-0" | 1'-0" | 1'-4" | 3'-2" | 5'-4" | 6'-1" | 1'-0" | 1'-6" | 3'-1" | 4'-7" | 6'-3" | - |
|  |  | 18' | 1'-0" | 1'-10" | 3'-8' | 5'-8" | $8^{\prime}-1{ }^{\prime \prime}$ | - | 2'-5" | 4'-0" | 5'-7" | 7'-1' | - | - |
|  |  | 22' | 2'-4" | 4'-1" | 6'-1" | 8'-3' | - | - | 4'-11" | 6'-6" | 8'-1" | 9'-9" | - | - |
|  |  | $26^{\prime}$ | $4^{\prime}-7^{\prime \prime}$ | 6'-6" | 8'-7' | 11'-0" | - | - | $7^{\prime}-5^{\prime \prime}$ | 9'-0" | 10'-7" | 12'-7" | - | - |
| $\begin{aligned} & \text { LPI 20Plus } \\ & \text { E } \\ & \text { LPI 32Plus } \end{aligned}$ | 9-1/2" | $6{ }^{\prime}$ | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 1'-5" | 1'-0" | 1'-0" | 1'-0" | 1'-3" | 1'-8" | 2'-0" |
|  |  | $10^{\prime}$ | 1'-0" | 1'-0" | 2'-6" | 2'-11" | 3'-5" | 3'-11" | 1'-0" | 2'-1" | $3^{\prime}-5^{\prime \prime}$ | 3'-9" | 4'-2' | - |
|  |  | $14{ }^{\prime}$ | 1'-7" | 3'-2' | 5'-0" | 5'-7" | 6'-1' | - | 3'-3' | 4'-7" | 5'-11" | 6'-5" | - | - |
|  |  | $18^{\prime}$ | 3'-11" | 5'-8" | 7'-9" | 8'-4" | - | - | 5'-9" | 7'-1" | - | - | - | - |
|  | 11-7/8" | $10^{\prime}$ | 1'-0" | 1'-0" | 1'-9" | 3'-3" | 3'-9" | 4'-3" | 1'-0" | 1'-9" | 2'-10" | 4'-0" | 4'-5" | - |
|  |  | $14{ }^{\prime}$ | 1'-5" | 2'-9" | 4'-2" | 5'-11" | 6'-6" | - | 3'-1' | 4'-3" | 5'-4" | - | - | - |
|  |  | 18' | 3'-8" | 5'-2" | 6'-9" | 8'-8" | - | - | 5'-7" | 6'-9" | 7'-11' | - | - | - |
|  |  | 22' | $6^{\prime}-1{ }^{\prime \prime}$ | 7'-9" | 9'-6" | - | - | - | 8'-1" | 9'-3" | - | - | - | - |
|  | 14" | $14{ }^{\prime}$ | 1'-0" | 1'-0" | 1'-0" | 2'-8" | 4'-11" | 5'-9" | 1'-0" | 1'-0" | 2'-6" | 4'-2" | 5'-10" | - |
|  |  | 18' | 1'-0" | 1'-0" | 2'-11" | 5'-1" | 7'-7" | 8'-6" | 1'-7" | $3{ }^{\prime}-3{ }^{\prime \prime}$ | 5'-0" | 6'-8" | - | - |
|  |  | 22' | 1'-4" | 3'-3' | 5'-4" | 7'-8" | 10'-5" | - | 4'-1" | 5'-9" | 7'-6" | 9'-2" | - | - |
|  |  | $26^{\prime}$ | 3'-6" | $5^{\prime}-7^{\prime \prime}$ | 7'-10" | 10'-4" | - | - | 6'-7" | 8'-3'' | 10'-0' | 12'-0" | - | - |
|  | 16" | 18' | 1'-0" | 1'-0" | 2'-5" | 4'-4" | 6'-5" | - | 1'-5" | 3'-0" | 4'-6" | 6'-1" | 7'-8" | - |
|  |  | 22' | 1'-2" | 2'-11" | 4'-9" | 6'-10" | 9'-2" | - | 3'-11' | 5'-6" | 7'-0" | 8'-7" | 10'-6" | - |
|  |  | $26^{\prime}$ | 3'-4' | 5'-2' | 7'-2" | 9'-5" | 11'-11" | - | 6'-5" | 8'-0" | 9'-6" | 11'-1" | - | - |
|  |  | $30^{\prime}$ | 5'-8" | 7'-7' | 9'-9" | 12'-1" | - | - | 8'-11" | 10'-6" | 12'-0" | 14'-0" | - | - |
| $\begin{gathered} \text { LPI } 36 \\ \text { G } \\ \text { LPI } 56 \end{gathered}$ | 11-7/8" | $10^{\prime}$ | 1'-0" | 1'-0" | 1'-9" | 3'-3' | 3'-9" | 4'-3" | 1'-0" | 1'-9" | 2'-10" | 4'-0" | 4'-5" | - |
|  |  | $14^{\prime}$ | 1'-5" | 2'-9" | 4'-2' | 5'-11" | 6'-6" | - | 3'-1' | 4'-3' | 5'-4" | - | - | - |
|  |  | 18' | 3'-8" | 5'-2' | 6'-9" | 8'-8" | - | - | 5'-7" | 6'-9" | 7'-11' | - | - | - |
|  |  | 22' | $6^{\prime}-1{ }^{\prime \prime}$ | 7'-9" | 9'-6" | - | - | - | $8^{\prime}-1{ }^{\prime \prime}$ | 9'-3' | - | - | - | - |
|  | 14" | $14^{\prime}$ | 1'-0" | 1'-0" | 1'-0" | 2'-8" | 4'-11" | 5'-9" | 1'-0" | 1'-0" | 2'-6" | 4'-2" | 5'-10" | - |
|  |  | 18' | 1'-0" | 1'-0" | 2'-11" | 5'-1' | 7'-7' | 8'-6" | 1'-7" | $3^{\prime}-3^{\prime \prime}$ | 5'-0' | 6'-8" | - | - |
|  |  | 22' | 1'-4" | $3{ }^{\prime}-3$ " | 5'-4" | 7'-8' | 10'-5" | - | 4'-1" | 5'-9" | 7'-6" | 9'-2" | - | - |
|  |  | $26^{\prime}$ | 3'-6" | 5'-7" | 7'-10" | 10'-4" | - | - | 6'-7" | 8'-3' | 10'-0" | 12'-0" | - | - |
|  | 16" | 18' | 1'-0" | 1'-0" | 2'-5" | 4'-4" | 6'-5" | - | 1'-5" | 3'-0" | 4'-6" | 6'-1" | 7'-8" | - |
|  |  | $22^{\prime}$ | 1'-2" | 2'-11" | 4'-9" | 6'-10" | 9'-2" | - | $3^{\prime}-11^{\prime \prime}$ | 5'-6" | 7'-0" | 8'-7" | 10'-6" | - |
|  |  | $26^{\prime}$ | 3'-4" | 5'-2" | 7'-2" | 9'-5" | 11'-11" | - | 6'-5' | 8'-0" | 9'-6" | 11'-1" | - | - |
|  |  | $30^{\prime}$ | 5'-8" | 7'-7' | 9'-9" | 12'-1" | - | - | 8'-11" | 10'-6" | 12'-0" | 14'-0" | - | - |
| LPI 42Plus <br> f <br> LPI 52Plus | 9-1/2" | $6{ }^{1}$ | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 1'-5" | 1'-0" | 1'-0" | 1'-0" | 1'-3" | 1'-8" | 2'-0" |
|  |  | 10' | 1'-0" | 1'-0" | 2'-6" | 2'-11" | 3'-5' | 3'-11' | 1'-0" | 2'-1" | 3'-5' | 3'-9" | 4'-2' | - |
|  |  | $14^{\prime}$ | 1'-7" | 3'-2" | 5'-0" | 5'-7" | 6'-1" | - | 3'-3' | 4'-7" | 5'-11" | 6'-5" | - | - |
|  |  | $18^{\prime}$ | 3'-11' | 5'-8" | 7'-9" | 8'-4" | - | - | 5'-9" | 7'-1" | - | - | - | - |
|  | 11-7/8" | $10^{\prime}$ | 1'-0" | 1'-0" | 1'-9" | 3'-3' | 3'-9" | 4'-3" | 1'-0" | 1'-9" | 2'-10" | 4'-0" | 4'-5" | - |
|  |  | $14^{\prime}$ | $1^{\prime}-5^{\prime \prime}$ | 2'-9" | 4'-2" | 5'-11" | 6'-6" | - | 3'-1' | 4'-3" | 5'-4' | - | - | - |
|  |  | 18' | 3'-8" | 5'-2" | 6'-9" | 8'-8" | - | - | 5'-7" | 6'-9" | 7'-11' | - | - | - |
|  |  | 22' | $6^{\prime}-1{ }^{\prime \prime}$ | 7'-9" | 9'-6" | - | - | - | 8'-1' | 9'-3' | - | - | - | - |
|  | 14" | $14^{\prime}$ | 1'-0" | 1'-0" | 1'-0" | 2'-8" | 4'-11" | 5'-9" | 1'-0" | 1'-0" | 2'-6" | 4'-2" | 5'-10" | - |
|  |  | 18' | 1'-0" | 1'-0" | 2'-11" | 5'-1" | 7'-7' | 8'-6" | 1'-7" | 3'-3' | 5'-0' | 6'-8" | - | - |
|  |  | 22' | 1'-4" | 3'-3" | 5'-4" | 7'-8" | 10'-5" | - | 4'-1" | 5'-9" | 7'-6" | 9'-2" | - | - |
|  |  | $26^{\prime}$ | 3'-6" | 5'-7" | 7'-10" | 10'-4" | - | - | 6'-7' | 8'-3' | 10'-0" | 12'-0" | - | - |
|  | 16" | 18' | 1'-0" | 1'-0" | 2'-5" | 4'-4" | 6'-5" | - | 1'-5" | 3'-0" | 4'-6" | 6'-1" | 7'-8" | - |
|  |  | $22^{\prime}$ | 1'-2" | 2'-11" | 4'-9" | 6'-10" | 9'-2" | - | $3^{\prime}-11^{\prime \prime}$ | 5'-6" | 7'-0" | 8'-7" | 10'-6" | - |
|  |  | 26' | 3'-4" | 5'-2" | 7'-2" | 9'-5" | 11'-11" | - | 6'-5" | 8'-0" | 9'-6" | 11'-1" | - | - |
|  |  | $30^{\prime}$ | 5'-8" | 7'-7" | 9'-9" | 12'-1" | - | - | 8'-11" | 10'-6" | 12'-0" | 14'-0" | - | - |

## DESIGN ASSUMPTIONS:

1. The hole locations listed above are valid for floor joists supporting only uniform loads. The total uniform load shall not exceed 130 plf (e.g., 40 psf Live Load and 25 psf Dead Load spaced 24" oc).
2. Hole location is measured from the inside face of bearing to the nearest edge of a rectangular hole, from the closest support.
3. Clear Span has not been verified for these joists and is shown for informational purposes only! Verify that the joist selected will work for the span and loading conditions needed before checking hole location.
4. The maximum hole depth for rectangular holes is the I-joist Depth less 4", except the maximum hole depth is $6^{\prime \prime}$ for $9-1 / 2^{\prime \prime}$ LPI joists, and $8^{\prime \prime}$ for $11-7 / 8^{\prime \prime}$ LPI Joists. Where the Maximum Hole Dimension exceeds the hole depth, the dimension refers to hole width and the depth of the hole is assumed to be the maximum for that joist depth. The maximum hole width is 18 ,' regardless of 1 -joist Depth.
5. Holes cannot be located in the span where designated "-", without further analysis by a design professional.

## NOTES

1. Holes may be placed anywhere within the depth of the joist. A minimum 1/4" clear distance is required between the hole and the flanges.
2. Round holes up to $1-1 / 2^{\prime \prime}$ diameter may be placed anywhere in the web.
3. Perforated "knockouts" may be neglected when locating web holes.
4. Holes larger than $1-1 / 2^{\prime \prime}$ are not permitted in cantilevers without special engineering.
5. Multiple holes shall have a clear separation along the length of the joist of at least twice the length of the larger adjacent hole, or a minimum of $12^{\prime \prime}$ center-to-center, whichever is greater
6. Multiple holes may be spaced closer provided they fit within the boundary of an acceptable larger hole. Example: two 3 " round holes aligned parallel to the joist length may be spaced 2" apart (clear distance) provided that a $3^{\prime \prime}$ high by $8^{\prime \prime}$ long rectangle or an 8 diameter round hole are acceptable for the joist depth at that location and completely encompass the holes.
7. For conditions not covered in this table, use LP's design software or contact your local $\mathrm{LP}^{\oplus}$ SolidStart ${ }^{\oplus}$ Engineered Wood Products distributor for more information


A3 BLOCKING AT EXTERIOR WALL


 to edge of plate, drive at an angle to reduce splitting


WEB STIFFENERS
AT INTERIOR SUPPORT
(When Required)


Verify stiffener requirements (see Web Stiffener detail)


LP SolidStart I-Joists shall be designed to carry all applied LP SolidStart 1-Joists shali be designed to carry all applied
loads including walls from above that do not stack directly over the I-Joist support.



## I-JOIST HEADER CROSS-SECTION

Web Filler/Backer Block: Backer blocks shall be at least 12" long and located behind every supported hanger. For a single I-joist header, install backer blocks to both sides of the web. Two pieces of $2 \times 8$ (min.) lumber, cut to the proper height (see notes 2 \& 3), may be set vertically side-by-side to achieve the required minimum 12 " length
Attach backer blocks with 8d nails (use 10d nails for flanges wider than 2-1/2"). Use a minimum of 10 nails spaced to avoid splitting, with half the nails to each side of the center of the supported hanger
Note: Backer blocks may be omitted for top-mount hangers supporting only downward loads not exceeding 250 lbs.
Filler Blocks: Install in minimum 4' long sections at each support, centered behind each supported hanger and at no more than $8^{\prime}$ oc. Lumber fillers may be stacked to achieve the required depth (see notes 2 \& 3). For example, two $4^{\prime}$ long $2 \times 8$ 's may be stacked vertically to achieve the filler depth for an 18" deep l-joist (minimum required depth is $18^{\prime \prime}-3^{\prime \prime}-1^{\prime \prime}=14^{\prime \prime}$ ).
Attach filler blocks with 8d nails (10d for flanges wider than 2-1/2") nails spaced $6^{\prime \prime}$ oc per row. Use one row of nails in each row of stacked fillers, with a minimum of two rows of nails. Drive every other nail from opposite sides.
NOTES:

1. Backer blocks and filler blocks shall consist of APA Rated wood structural panel (OSB or plywood), $2 \times$ lumber (SPF or better), or LP® SolidStart ${ }^{\ominus}$ LVL, LSL or OSB Rim Board, with a net thickness equivalent to that shown in the l-Joist Filler Thickness table below.
2. Except as noted in 3 , backer blocks and filler blocks shall fit the clear distance between flanges with a gap of at least $1 / 8$, " but not more than 1, " and shall be of sufficient depth to allow for all hanger nailing into the web. Do not force into place.
3. Backer blocks and filler blocks for double I-joists that are top-loaded only or side-loaded supporting top-mount hangers that do not require nailing into the web, shall be at least $5-1 / 2^{\prime \prime}$ deep for 1 -joists to $11-7 / 8^{\prime \prime}$ deep, and shall be at least $7-1 / 4^{\prime \prime}$ deep for 1 -joists 14 " and deeper.
4. Install backer blocks tight to top flange for top-loaded joists and for joists supporting top-mount hangers (shown). Install tight to bottom flange for joists supporting face-mount hangers.
5. Clinch nails where possible.
6. For double I-joists, additional nailing may be required to transfer point loads. For additional information, contact your LP SolidStart Engineered Wood Products distributor.


| I-JOIST FILLER THICKNESS |  |  |
| :---: | :---: | :---: |
| Series | Filler Block | Web Filler/Backer Block |
| LPI 18 <br> LPI 20Plus <br> LPI 32Plus | $2-1 / 8^{\prime \prime}$ | $1{ }^{\prime \prime}$ |
| LPI 36 | $1-7 / 8^{\prime \prime}$ | $7 / 8^{\prime \prime}$ |
| LPI 42Plus <br> LPI 52Plus <br> LPI 56 | $3 "$ | $1-1 / 2^{\prime \prime}$ |

## NOTES:

1. Backer blocks and filler blocks shall consist of APA Rated wood structural panel (OSB or plywood), or $2 x$ lumber (SPF or better).
2. LP LVL, LSL or OSB Rim Board may also be used.
3. Refer to the Notes for the I-Joist Header Cross-Section above for details on the required height and length, and nailing of the backer blocks and filler blocks.
bevel cut/fire cut
LPI blocking or other lateral support required at ends of I-Joist


## GENERAL NOTES:

1. Some wind or seismic loads may require different or additional details and connections. 2. Verify building code requirements for suitability of details shown.
2. Refer to page 4 for bearing length requirements.
3. Refer to page 28 for Flange Face Nailing Schedule for LPI rim joist or blocking panel nailing
4. Lateral support shall be considered for bottom flange when there is no sheathing on underside.
5. Verify capacity and fastening requirements of hangers and connectors.
6. Squash block capacity designed by others.
7. Do not use LPI joists with flanges wider than $2-1 / 2^{\prime \prime}$ as rim joists.

## Roof Details



## NOTES:

1. Minimum pitch: $1 / 4^{\prime \prime}$ per foot $(1 / 4: 12)$.

Maximum pitch: 12" per foot (12:12).
2. Verify capacity and fastening requirements of hangers and connectors.
3. Some wind or seismic loads may require different or additional details and connections. Uplift anchors may be required.
4. 4" diameter hole(s) may be cut in blocking for ventilation.
5. Lateral resistance shall be provided. Other methods of restraint, such as full depth LP SolidStart OSB Rim Board, LP ${ }^{\oplus}$ SolidStart ${ }^{\oplus}$ LVL, LP SolidStart LSL or metal X-bracing may be substituted for the LP blocking shown.

## GENERAL NOTES

1. The following tables provide a list of the more common hangers and connectors for use with LP® SolidStart ${ }^{\ominus}$--Joists
2. Refer to the manufacturer's connector guide for a complete list of hangers and to verify the suitability of a hanger or connector for a particular application.
3. Follow all connector manufacturers' installation guidelines.

SIMPSON STRONG-TIE ${ }^{\bullet}$

| Series | Depth | Top-Mount |  | Face-Mount |  | $45^{\circ}$ Skewed | Field Slope \& Skew | Variable Pitch Seat |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Single | Double | Single | Double | Single | Single | Single |
| $\begin{gathered} \text { LPI } 18 \\ \text { LPI 20Plus } \\ \text { LPI 32Plus } \end{gathered}$ | 9-1/2" | ITS2.56/9.5 | MIT39.5-2 | IUS2.56/9.5 | MIU5.12/9 | SUR/L2.56/9 | LSSUH310 * | VPA3 |
|  | 11-7/8" | ITS2.56/11.88 | MIT311.88-2 | IUS2.56/11.88 | MIU5.12/12 | SUR/L2.56/11 | LSSUH310* | VPA3 |
|  | 14" | ITS2.56/14 | MIT314-2 | IUS2.56/14 | MIU5.12/14 | SUR/L2.56/14 | LSSUH310 * | VPA3 |
|  | $16^{\prime \prime}$ | ITS2.56/16 | MIT5.12/16 | IUS2.56/16 | MIU5.12/16 | SUR/L2.56/14 * | ** | VPA3 |
| LPI 36 | 11-7/8" | ITS2.37/11.88 | MIT3511.88-2 | IUS2.37/11.88 | MIU4.75/11 | SUR/L2.37/11 | LSSUI35 * | VPA35 |
|  |  | ITS2.37/14 | MIT3514-2 | IUS2.37/14 | MIU4.75/14 | SUR/L2.37/14 | LSSUI35 * | VPA35 |
|  | 16" | ITS2.37/16 | MIT4.75/16 | IUS2.37/16 | MIU4.75/16 | SUR/L2.37/14 * | ** | VPA35 |
| LPI 42Plus LPI 52Plus LPI 56 | 9-1/2" | ITS3.56/9.5 | B7.12/9.5 * | IUS3.56/9.5 | HU410-2 * | SUR/L410* | LSSU410 * | VPA4 |
|  | 11-7/8" | ITS3.56/11.88 | B7.12/11.88 * | IUS3.56/11.88 | HU412-2 * | SUR/L410 * | LSSU410 * | VPA4 |
|  | 14 " | ITS3.56/14 | B7.12/14 * | IUS3.56/14 | HU414-2 * | SUR/L414* | LSSU410 * | VPA4 |
|  | 16" | ITS3.56/16 | B7.12/16 * | IUS3.56/16 | HU414-2 * | SUR/L414* | ** | VPA4 |

Web filler required for proper installation of hanger.
** Refer to Simpson Strong-Tie "Wood Construction Connectors" catalog for hanger selection.

## USP STRUCTURAL CONNECTORS®

| Series | Depth | Top-Mount |  | Face-Mount |  | $45^{\circ}$ Skewed | Field Slope \& Skew | Variable Pitch Seat ${ }^{1}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Single | Double | Single | Double | Single | Single | Single |
| LPI 18 LPI 20Plus LPI 32Plus | 9-1/2" | TFL2595 | TH025950-2 * | THF25925 | THF25925-2 * | SKH2520L/R * | LSSH25 * | TMP25 or TMPH25 * |
|  | 11-7/8" | TFL25118 | TH025118-2 * | THF25112 | THF25112-2 * | SKH2520L/R * | LSSH25 * | TMP25 or TMPH25 * |
|  | $14 "$ | TFL2514 | TH025140-2 * | THF25140 | THF25140-2 * | SKH2524L/R * | LSSH25 * | TMP25 or TMPH25 * |
|  | 16" | TFL2516 | THO25160-2 * | THF25160 | THF25160-2 * | SKH2524L/R * | LSSH25 * + | TMP25 or TMPH25 * |
| LPI 36 | 11-7/8" | TFL23118 | THO23118-2 * | THF23118 | THF23118-2 * | SKH2320L/R * | LSSH23 * | TMP23 or TMPH23 * |
|  | 14" | TFL2314 | TH023140-2 * | THF23140 | THF23140-2 * | SKH2324L/R * | LSSH23 * | TMP23 or TMPH23 * |
|  | 16" | TFL2316 | TH023160-2 * | THF23160 | THF23160-2 * | SKH2324L/R * | LSSH23 * + | TMP23 or TMPH23 * |
| LPI 42Plus LPI 52Plus LPI 56 | 9-1/2" | TH035950 | BPH7195 * | THF35925 | HD7100 * | SKH410L/R * ** | LSSH35 * | TMP4 or TMPH4 * |
|  | 11-7/8" | TH035118 | BPH71118 * | THF35112 | HD7120 * | SKH410L/R *** | LSSH35 * | TMP4 or TMPH4 * |
|  | 14" | TH035140 | BPH7114 * | THF35140 | HD7140 * | SKH414L/R *** | LSSH35 * | TMP4 or TMPH4 * |
|  | 16" | TH035160 | BPH7116 * | THF35157 | HD7160 * | SKH414L/R *** | LSSH35 * + | TMP4 or TMPH4 * |

[^5]


WEB STIFFENER REQUIREMENTS

| Series | Depth | Minimum Thickness | Maximum Height | Nail Size* | Nail Qty |
| :---: | :---: | :---: | :---: | :---: | :---: |
| LPI 18 LPI 20Plus LPI 32Plus | 9-1/2" | 23/32" | 6-3/8" | 8d (2-1/2") | 3 |
|  | 11-7/8" | 23/32" | 8-3/4" | 8d (2-1/2") | 3 |
|  | 14" | 23/32" | 10-7/8" | 8d (2-1/2') | 3 |
|  | 16 " | 23/32" | 12-7/8" | 8d (2-1/2') | 3 |
| LPI 36 | 11-7/8" | 23/32" | 8-3/4" | 8d (2-1/2") | 4 |
|  | 14" | 23/32" | 10-7/8" | 8d (2-1/2") | 5 |
|  | 16" | 23/32" | 12-7/8" | 8d (2-1/2") | 6 |
| LPI 42Plus <br> LPI 52Plus | 9-1/2" | 1-1/2" | 6-3/8" | 10d (3") | 3 |
|  | 11-7/8" | 1-1/2" | 8-3/4" | 10d (3") | 3 |
|  | 14" | 1-1/2" | 10-7/8" | 10d (3") | 3 |
|  | 16 " | 1-1/2" | 12-7/8" | 10d (3") | 3 |
| LPI 56 | 11-7/8" | 1-1/2" | 8-3/4" | 10d (3") | 4 |
|  | 14 " | 1-1/2" | 10-7/8" | 10d (3") | 5 |
|  | 16" | 1-1/2" | 12-7/8" | 10d (3") | 6 |

* Nails may be Box or Common.

| Series | Depth | Uniform Vertical Load Capacity |
| :---: | :---: | :---: |
|  |  | (plf) |
| $\begin{aligned} & \text { LPI } 18 \\ & \text { LPI 20Plus } \end{aligned}$ | 9-1/2" | 1900 |
|  | 11-7/8" | 1760 |
|  | 14 " | 1600 |
| LPI 20Plus | 16 " | 1500 |
| LPI 32Plus <br> LPI 42Plus | 9-1/2" | 2200 |
|  | 11-7/8" | 2200 |
|  | 14" | 1600 |
|  | 16" | 1500 |
| LPI 36 | 11-7/8" | 1800 |
|  | 14" | 1800 |
|  | 16 " | 1800 |
| LPI 52Plus LPI 56 | 11-7/8" | 2400 |
|  | 14" | 2200 |
|  | 16 " | 1900 |

## NOTES:

1. Uniform Vertical Load Capacity shall not be adjusted for load duration.
2. Concentrated vertical loads require the addition of squash blocks. Do not use LPI rim or blocking to support concentrated vertical loads.
3. Lateral load capacity for all series above is 200 plf but may be limited by the connection details used. Do not exceed the Flange Face Nailing requirements at right.

| FLANGE FACE NAILING |  |  |  |
| :---: | :---: | :---: | :---: |
| Series | Nail Size and Type | Minimum Nail Distance |  |
|  |  | oc Spacing | End |
| LPI 18 LPI 20Plus <br> LPI 32Plus <br> LPI 42Plus <br> LPI 52Plus | 8d (2-1/2") Box or Common | 2" | $1{ }^{\prime \prime}$ |
|  | 10d (3") or 12d (3-1/4") Box | 2" | $1{ }^{\prime \prime}$ |
|  | 10d (3") or 12d (3-1/4") Common | $3 "$ | 1-1/2" |
|  | 16d Sinker (3-1/4") | $3 "$ | 1-1/2" |
|  | 16d (3-1/2") Box or Common | 4" | 1-1/2" |
| LPI 36 <br> LPI 56 | 8d (2-1/2") Box or Common | $3{ }^{\prime \prime}$ | 1-1/2" |
|  | 10d (3") or 12d (3-1/4") Box | $3{ }^{\prime \prime}$ | 1-1/2" |
|  | 10 d (3") or 12d (3-1/4") Common | $3{ }^{\prime \prime}$ | 1-1/2" |
|  | 16d Sinker (3-1/4") | $3 "$ | 1-1/2" |
|  | 16d (3-1/2") Box or Common | 5" | 1-1/2" |

## NOTES:

1. Use only 10d box or 8d nails when securing an LPI floor or roof joist to its supports.

## RIM BOARD CAPACITIES

| Material | Grade | Thickness | Vertical Load Capacity ${ }^{1}$ |  |  | Latera ${ }^{4,5,5}$ Load Capacity (plf) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $\begin{aligned} & \text { Uniform}^{2} \\ & \text { (plf) } \end{aligned}$ |  | Concentrated ${ }^{3}$ (Ibs) |  |
|  |  |  | d $\leq 16{ }^{\prime \prime}$ | $16^{\prime \prime}<\mathrm{d} \leq 24{ }^{\prime \prime}$ | d $\leq 24{ }^{\prime \prime}$ |  |
| LP OSB | APA Rated | $1{ }^{1 \prime}$ | 3300 | 1650 | 3500 | 180 |
| LP OSB | Rim Board | 1-1/8" | 4400 | 3000 | 3500 | 180 |
| LP LSL | $1730 \mathrm{~F}_{\mathrm{b}}-1.35 \mathrm{E}$ | $\geq 1-1 / 4^{\prime \prime}$ | 6000 | 3800 | 3800 | 250 |
| LP LVL (cross-ply) | $1750 \mathrm{~F}_{\mathrm{b}}-1.3 \mathrm{E}$ | $\geq 1-1 / 4^{\prime \prime}$ | 9350 | 5070 | 4210 | 250 |

## NOTES:

1. The Vertical Load Capacity shall not be increased for short-term load duration
2. The Vertical Load Capacity is based on the capacity of the rim board and may need to be reduced based on the bearing capacity of the supporting wall plate or the attached floor sheathing. Example: The allowable bearing stress for commodity floor sheathing is 360 psi so the bearing capacity of a $1-1 / 4^{\prime \prime} \times 16^{\prime \prime}$ deep rim board would be limited to 5400 plf ( $360 \mathrm{psi} \times 1-1 / 4^{\prime \prime} \times 12$ ).
3. The Concentrated Vertical Load capacity is assumed to be applied through a minimum 4-1/2" bearing length ( 3 -stud post).
4. The Lateral Load Capacity is based on a short-term load duration and shall not be increased.
5. The Lateral Load Capacity is based on the connections specified in the Installation details below.
6. Additional framing connectors fastened to the face of the rim board may be used to increase lateral capacity for wind and seismic design.

## ALLOWABLE UNIFORM LOADS (PLF) FOR OSB RIM BOARD HEADERS: MAXIMUM 4' CLEAR SPAN

| Material | Thickness | Rim Board Depth |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 9-1/2" | 11-7/8" | 2-Ply 14" | 2-Ply 16" |
| LP OSB | $1{ }^{\prime \prime}$ | 330 (1-1/2") | 480 (3") | 1280 (3") | 1670 (4-1/2") |
|  | 1-1/8" | 370 (1-1/2") | 540 (3") | 1440 (3') | 1880 (4-1/2") |
| LP LSL | 1-1/4" | 655 (1-1/2") | 1240 (3") | 3540 (4-1/2") | 4485 (6") |
| LP LVL (cross-ply) | 1-1/4" | 595 (1-1/2") | 1125 (3") | 3210 (4-1/2") | 4065 (6") |

## NOTES:

1. This table is for preliminary design for uniform gravity loads only. Final design should include a complete analysis of all loads and connections.
2. The allowable loads are for a maximum 4' clear span with minimum bearings for each end (listed in parentheses) based on the bearing capacity of the rim board. For headers bearing on wood plates, the bearing length may need to be increased based on the ratio of the bearing capacity of the rim board divided by the bearing capacity of the plate species.
3. Normal load duration is assumed and shall be adjusted according to code.
4. Depths greater than 11-7/8" shall be used with a minimum of two plies, as shown. Depths of 11-7/8" and less may be used as a two-ply header by multiplying the allowable loads by two.
5. Multiple-ply headers shall be toe-nailed to the plate from both faces. Fasten the floor sheathing to the top of each ply to provide proper lateral support for each ply.
6. For multiple-ply headers supporting top-loads only, fasten plies together with minimum 8 d box nails ( $2-1 / 2^{\prime \prime} \times 0.113^{\prime \prime}$ ) at a maximum spacing of $12^{\prime \prime}$ oc. Use 2 rows of nails for $9-1 / 2^{\prime \prime}$ and 11-7/8." Use 3 rows for depths 14 " and greater. Clinch the nails where possible. For side-loaded multiple-ply headers, refer to the Connection Capacity For Side-Loaded 2-Ply Rim Board Headers table below for the required nailing and the allowable side load that can be applied.
7. The designer shall verify proper bearing for the header.
8. Joints in the rim are not allowed over openings and must be located at least 12 " from any opening.
9. Refer to the "APA Performance Rated Rim Boards" (Form No. W345) for additional information including allowable loads for smaller openings.
10. Use LP® SolidStart ${ }^{\oplus}$ LSL or LVL for headers with clear spans longer than 4 ' or for loads greater than tabulated above. Refer to the appropriate technical guide for LP LSL and LVL design values.

## CONNECTION CAPACITY FOR SIDE-LOADED 2-PLY RIM BOARD HEADERS (PLF)

| Material | Thickness | Minimum <br> Nail Size | 3 Rows of Nails <br> at 6" oc | 4 Rows of Nails <br> at 6" oc | 5 Rows of Nails <br> at 6" oc |
| :---: | :---: | :---: | :---: | :---: | :---: |
| LP OSB | $1 " \& 1-1 / 8^{\prime \prime}$ | $8 \mathrm{~d}\left(2-1 / 2^{\prime \prime} \times 0.113^{\prime \prime}\right)$ | 768 | 1024 | 1280 |
| LP Rows of Nails <br> at 6" oc |  |  |  |  |  |
| LP LVL (cross-ply) | $1-1 / 2^{\prime \prime} \& 1-3 / 4^{\prime \prime}$ | $10 \mathrm{~d}\left(2-1 / 2^{\prime \prime} \times 0.113^{\prime \prime}\right)$ | 864 | 1536 |  |

## NOTES:

1. This table represents the uniform side-load capacity of the connection for a 2 -ply header. The total applied uniform load, including top-load and side-load, shall not exceed the allowable uniform load capacity of the header as tabulated above.
2. The tabulated side-load capacity is for normal load duration and shall be adjusted according to code.
3. Use 3 rows of nails for $9-1 / 2^{\prime \prime}$ and $11-7 / 8^{\prime \prime} ; 4$ rows for $14^{\prime \prime}$ and $16^{\prime \prime} ; 5$ rows for $18^{\prime \prime}$ and $20 " ; 6$ rows for $22^{\prime \prime}$ and $24^{\prime \prime}$ deep rim board. Clinch the nails where possible.
4. Headers consisting of more than 2 plies, alternate fastening or higher side loads are possible but require proper design of the connection.

## INSTALLATION



## T\&G TRIM REQUIREMENTS ${ }^{2}$

See TGG Trim Requirements table below for when to trim tongue or groove.


NOTE:

1. Additional framing connectors to the face of the rim board may be used to increase lateral capacity for wind and seismic design.
2. Trim the tongue or groove of the floor sheathing in accordance with the T\&G Trim Requirements table.

## T\&G TRIM REQUIREMENTS

| Floor Sheathing <br> Thickness | Rim Board Thickness |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | $\mathbf{1 " ~}^{\prime \prime}$ | $\mathbf{1 - 1 / 8 "}$ | $\mathbf{1 - 1 / 4 "}$ | $>1-1 / \mathbf{4}^{\prime \prime}$ |
| $\leq 7 / 8^{\prime \prime}$ | Trim | Not Required | Not Required | Not Required |
| $>7 / 8^{\prime \prime}$ | Trim | Trim | Trim | Not Required |

- Warning: Failure to follow good procedures for handling, storage and installation could result in unsatisfactory performance, unsafe structures and possible collapse.
- Keep LP ${ }^{\oplus}$ SolidStart ${ }^{\oplus}$ Engineered Wood Products dry.
- Unload products carefully, by lifting. Support the bundles to reduce excessive bowing. Individual products shall be handled in a manner which prevents physical damage during measuring, cutting, erection, etc. I-Joists shall be handled vertically and not flatwise.
- Keep products stored in wrapped and strapped bundles, stacked no more than 10' high. Support and separate bundles with $2 \times 4$ (or larger) stickers spaced no more than 10' apart. Keep stickers in line vertically.
- Product shall not be stored in contact with the ground, or have prolonged exposure to the weather.
- Use forklifts and cranes carefully to avoid damaging products.
- Do not use a visually damaged product. Call your local LP SolidStart Engineered Wood Products distributor for assistance when damaged products are encountered.
- For satisfactory performance, LP SolidStart Engineered Wood Products shall be used under dry, covered and well-ventilated interior conditions in which the equivalent moisture content in lumber will not exceed 16\%.




## W ARNINGS

## The following conditions are NOT permitted!

Do not use visually damaged products without first checking with your local LP SolidStart Engineered Wood Products distributor or sales office.
DON'T put holes too close
to supports.
DON'T overcut hole
and damage flange.

## LP SolidStart

For more information on the full line of LP® SolidStart Engineered Wood Products or the nearest distributor, visit our web site at LPCorp.com.
Phone: 1-888-820-0325
E-mail: customer.support@LPCorp.com.
LP SolidStart Engineered Wood Products are manufactured at different locations
in the United States and Canada. Please verify availability with the LP SolidStart
Engineered Wood Products distributor in your area before specifying these products.

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[^0]:    * Deflections rounded to the nearest $1 / 16$.'

[^1]:    * Deflections rounded to the nearest $1 / 16$."

[^2]:    * Deflections rounded to the nearest $1 / 16$."

[^3]:    * Deflections rounded to the nearest $1 / 16$.'

[^4]:    Since the Total Joist Reaction, 2656 Ibs., is greater than the End Reaction Capacity w/out Stiffeners, 1122 Ibs., and is greater than the End Reaction Capacity with Stiffeners, 1443 Ibs., and is greater than the End Reaction Capacity with Web Filler Reinforcing, 2600 Ibs., but is less than the End Reaction Capacity with 23/32" APA Rated Sheathing, 3500 lbs., this joist requires the installation of Full-Depth Reinforcing One Side (detail C8) w/ 23/32" APA Rated Sheathing at the bearing.

[^5]:    * Web filler required for proper installation of hanger.
    ** Miter cut required on end of joist.
    $\dagger$ Hanger height is less than $60 \%$ of the joist depth. Supplemental lateral support of the top flange is required. Refer to USP's installation instructions.

    1. Use TMP seats for joist pitch of 1:12 to 6:12. Use TMPH for joist pitch of 6:12 and greater.
